

1997 HBP QC For Pay Pilot Projects With Void Acceptance

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Interim Report May, 1998

Prepared in Cooperation with the U.S. Department of Transportation Federal Highway Administration

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REPORT DOCUMENTATION PAGE FORM APPROVED

			OMB NO. 0704-0188
gathering and maintaining the data needed, ar	nd completing and reviewing the collection of inf	sponse, including the time for reviewing instruction of the comments regarding this burden quarters Services, Directorate for Information Opensia.	estimate or any other aspect of this
		Budget, Paperwork Reduction Project (0704-01)	
1. AGENCY USE ONLY (Leave Blank)	2. REPORT DATE	3. REPORT TYPE AND DATES COV	ERED
	May, 1998	Interim Report 1997	
4. TITLE AND SUBTITLE			5. FUNDING NUMBERS
1997 HBP QC For Pay Pilot Proje	ects with Void Acceptance		
6. AUTHORS(S)			
Bud A. Brakey			
7. PERFORMING ORGANIZATION N.	• • • • • • • • • • • • • • • • • • • •		8. PERFORMING ORGANIZATION
Colorado Department of	Transportation		REPORT NUMBER
4201 E. Arkansas Ave.			CDOT-DTD-R-98-3
Denver, Colorado 8022	2		
9. SPONSORING/MONITORING AGE			10. SPONSORING/MONITORING
Colorado Department of	Transportation		AGENCY REPORT NUMBER
4201 E. Arkansas Ave.			CDOT-DTD-R-98-3
Denver, Colorado 8022	2		
11. SUPPLEMENTARY NOTES			
Prepared in Cooperation	n with the U.S. Department	of Transportation, Federal	
Highway Administration	1		
12a. DISTRIBUTION/AVAILABILITY	STATEMENT		12b. DISTRIBUTION CODE
No Restrictions: This r	eport is available to the pub	lic through the	
National Technical Info	rmation Service Springfiel	d, VA 22161	
13. ABSTRACT (Maximum 200 words)			
CDOT's first two HBP pro	pjects using contractors' qua	ality control (QC) tests as ba	sis of payment. QC tests for
void analysis were required	d. Two levels of verification	were used: Results of rando	om split samples (VT's) by
		ctor VT's were compared to	
were by statistical F-test ar	nd T-test. Conclusions: Co	ntractors VT sets were with	in limits. The procedures
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can safely be 1 out of 8. G	ive more weight to air void	s - less to VMA in pay calcu	lations. Consider voids
filled with asphalt when ma	iking changes in the job mix	formula. Implementation	: Use F-test & T-test to
verify that contractor's QC	tests for pay are accurate a	nd unbiased. Give more wei	ght to AV and less to VMA
in composite PF formula.	More training is required for	r industry and CDOT on vol	umetric property testing
Consider effects on VFA w	vhen making field changes in	the job mix formula.	r
14. SUBJECT TERMS			15. NUMBER OF PAGES
	, Volumetric, Acceptance, Voids, VI	MA, VFA, Quality Level, Analysis	
Tolerances			16. PRICE CODE
17. SECURITY CLASSIFICATION	18. SECURITY CLASSIFICATION	19. SECURITY CLASSIFICATION	20. LIMITATION OF ABSTRACT

OF ABSTRACT

Unclassified

OF THIS PAGE

Unclassified

OF REPORT

Unclassified

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1997 HOT BITUMINOUS PAVEMENT QC FOR PAY PILOT PROJECTS, WITH VOID ACCEPTANCE

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1997 HOT BITUMINOUS PAVEMENT QC FOR PAY PILOT PROJECTS, WITH VOID ACCEPTANCE

BACKGROUND OF QC FOR PAY AND VOID ACCEPTANCE

CDOT began their quality control and quality assurance (QC&QA) program for hot bituminous pavements (HBP) in 1992 when they began a three-year pilot program. It was essentially completed in 1994, but a few projects were held over and completed in 1995. The Pilot specification computer software was designated QPM 1⁽¹⁾; also, the term used herein to identify that series of projects. In 1994 a revised, updated specification, designated as QPM 2⁽²⁾ was written. It was used on several projects completed in 1995 and all regular HBP projects completed in 1996 through 1998. Reports have been written for each of the six QC&QA years ^(3 to 8), 1992 through 1997, and are available from the CDOT library.

A long-range goal of the QC&QA program was to base contract payment on Contractors' QC tests. After five years in the program, most involved personnel believe QC tests reliably reflect the quality of construction, just as CDOT's QA tests do. This being the case, QC tests should be satisfactory for pay calculations. Where used for pay, QC tests must be randomly verified by CDOT to assure they are accurate and unbiased. By adopting QC tests for pay, a reduction in CDOT field testing should be possible. On Federal Aid projects, regulations permit QC for pay (QCFP), provided certain guidelines are met. In 1996, a concerted effort was made by CDOT and industry people, with support from FHWA representatives, to develop a pilot QCFP specification for HBP.

During the period, 1992 to 1996, many rapid changes were taking place in asphalt pavement mix design and construction technology. CDOT committed to keep up with technology changes. They concentrated on two major advances: (1) Adoption of the Superpave (SP)⁽⁹⁾ mix-design procedure and (2) Voids acceptance (VA) of field mixtures based on the laboratory volumetric properties during construction. Under VA, asphalt content and in-place density remains as acceptance elements, but *field* acceptance of gradation is dropped. A pilot VA program began in 1992, and by 1996, nine projects had been completed (10) and reported. Three more were completed in 1997 and reported (11). At the end 1996, only five SP projects had been completed, including three VA/SP projects. In 1997, 44 of 57 QC&QA jobs advertised for bid were **full** SP projects, including performance graded (PG) asphalt cement **and** SP aggregate grading designations.

While developing the pilot QCFP specification (in late 1996), some CDOT engineers wanted to combine the three technologies, QCFP, VA and SP design, into a single pilot specification. Industry members and others expressed concern over this approach, fearing this would introduce too many new things at once. Contractors, and private laboratories, were just beginning to get SP compactors and had yet to do any significant amount of field control testing for VA. Until then, CDOT did all field testing for voids properties on VA projects. Their tests were being used for plant control *and* acceptance. CDOT was just completing the switch from the Texas Gyratory to the SP compactor for mix design. Under the QCFP concept, for the first time, contractors would be required to make QC tests for voids properties. In addition, the new SP lab compactor would be specified along with PG asphalts and SP gradations.

CDOT, with assistance from industry, addressed the various concerns and wrote a pilot QCFP specification (Exhibit 1, attached) for the 1997 construction season. Standard QC&QA HBP specifications ⁽²⁾ were modified to make QC tests (instead of CDOT acceptance) the basis of payment for the usual three elements, asphalt content, in-place density and gradation. Contractors have plenty of experience making QC tests on these three elements and did not foresee problems here. However, they were concerned about doing percent air voids (AV) and voids in the mineral aggregate (VMA) tests for pay. This stemmed from their lack of familiarity with SP and VA test procedures. CDOT addressed this in the pilot by not assessing *disincentive* payments for Pay Factors of less than 1.0 for voids properties. Adjustments were required to bring properties within acceptable limits, however. As motivation, a special *incentive* pay formula, based on quality level analysis (QLA) of VMA and VA, was included.

THE QCFP PILOT SPECIFICATION

CDOT's standard QC&QA specifications ⁽²⁾ have stringent requirements for the contractors' QC program. The QCFP pilot maintained these requirements. In addition, a procedure was included for verification testing to assure QC tests would correctly represent the work. During development of the QCFP specification, plenty of discussion took place about the number of CDOT verification tests to be taken in relation to the number of QC tests. To avoid duplication of effort, only the absolute minimum number of verification tests should be taken. However, in this first effort, the ratio of CDOT tests to QC was kept high for a greater level of confidence and to provide more data for post construction analysis.

Under QC&QA specifications, all contractor and CDOT tests are randomly selected. For this pilot, one verification test (VT) is randomly selected by CDOT from each defined element strata (from the stratified random sampling schedule). Each VT is split and tested by the contractor and CDOT (for inplace nuclear density, the same spot is tested by each) and reported to the engineer. The sampling ratio of VT to QC ranges from 1:1 for in-place density, to 1:3 for gradation and voids properties, to 1:7 for asphalt content.

Standard statistical *F-test* and *t-test* procedures ⁽¹²⁾ are used to verify that the various sets of test results are statistically similar within defined probabilities. The *F-test* provides a method for comparing the variances (standard deviation [SD] squared) of two sets of data. Differences in means are evaluated by the *t-test*. Comparisons in the field are continuous, as results became available.

CDOT developed a spreadsheet computer program that does the calculations and provides the results as test results are entered accumulatively for each element. Two sets of test values from the same process obtained by nearly identical procedures will usually have different means and standard deviations. Such differences can be by random chance alone. The program calculates means and variances, then determines the probabilities that the two sets of data are tested by the same procedure. Large probability numbers, up to 100%, show good agreement in sampling and testing procedures on similar materials. When probabilities are low (1% or 0.5%) that the differences are not by random chance, flawed procedures may have been used in obtaining one or both data sets. If this happens, specified actions are required by the engineer.

The *F-test* and *t-test* are used to compare contractor VT's with CDOT VT's on running five-sample splits. As a check testing program, the first five VT pairs are compared and must be acceptable before the work can continue. If not acceptable, the check testing phase continues, after corrective actions, until the evaluation shows acceptable results. Two levels of probabilities are used, 5% or less warns of potential problems and 1% or less, requires corrective actions.

During routine production, the contractor's VT's are compared with the rest of the QC tests from which the VT's were randomly selected. Again, two levels of probabilities are used, 5% or less gives a warning, 0.5% or less, is not acceptable. A running, accumulated calculation is made for information, but the acceptance decision is made only when all tests have been completed. If the comparison is acceptable, the contractor is paid, incentive or disincentive, based on quality level analysis ⁽¹³⁾ (QLA) of all QC tests (including contractors' VT's). If they are not acceptable, payment is based on CDOT's VT results.

THE 1997 PILOT QC FOR PAY PROGRAM

Initially, CDOT hoped that each region would let two QCFP projects for the 1997 season. After further evaluation, management determined that SP adoption had the highest priority and efforts would be concentrated there. However, to start the QCFP program, Region 6 volunteered two projects.

The projects, (1) I 25, Hampden - South, and (2) Colorado Boulevard, Mississippi Avenue to Martin Luther King Boulevard, were let to contract and completed by fall. A single contractor, Kiewit Western Company was the successful bidder on both projects. This report summarizes the data provided by the contractor and CDOT field personnel. Particular emphasis has been placed on areas where the greatest concerns were expressed during development of the QCFP pilot.

The QCFP program will pause for 1998. It is expected to resume in 1999 under a revised QCFP specification (now being written) that will incorporate the three technologies referred to above, namely QCFP, VA and SP. In 1998, the VA pilot program (using full SP) will continue at an increased rate under a revised specification, now in use, (see Reference 11, Exhibit 1) that closely parallels QPM 2. This specification requires full QC testing for volumetric properties. As with previous VA projects, field acceptance testing of aggregates is not included. However, QC sieve analysis testing and reporting are required by the contractor, but not for pay. The specification evaluates four elements by QLA. PF's are calculated by the same formulas as in QPM 2. As in the previous VA pilots, the element "W" factors are 0.1 for AC%, 0.4 for density, 0.2 for VMA and 0.3 for AV. The QPM 2 "W" factors are 0.3 for AC%, 0.5 for density and 0.2 for gradation. "W" is a relative weighting factor applied to the element PF's when calculating the item composite PF.

SUMMARY AND ANALYSIS OF THE 1997 OCFP DATA

The first page of Table 1 separately lists the field data from the two projects. Columns headings identify data in rows across from the listings at left. Cells are shaded if not applicable. Data is not available where "NA" shows. Elements in each process are grouped with the normal QPM 2 elements listed first,

followed by AV and VMA, as pay elements. Percent voids filled with asphalt (VFA) has been added for information.

The SP procedure by CDOT includes VFA with a *design* parameter of 65-75 for medium-to-heavy traffic. Not specified for the projects, but used here to calculate QL, was a target of 70 and a tolerance of 7.0. CDOT has elected not to specify VFA as field acceptance criteria, because it is redundant. VFA = [(VMA - AV)/VMA] x 100. It is controlled by adhering to the target of 4% air voids and the specified minimum VMA. AV and VMA have a linear relationship to VFA. Figure 1 shows this where the VMA target is 14.0 (lowest *design* target allowed for grading S) and the AV target is 4.0. For these targets, if production is controlled such that the PF is maintained at 1.0, or higher, ("n" = 15), there is only a slight possibility of VFA being outside the recommended range. The effect of varying or maintaining AC% is not shown in Figure 1. Field adjustments to the job-mix formula can easily cause VFA to rise above 75. For mix designs and checks, the Central Laboratory routinely calculates and reports VFA. In Table 1, VFA has been calculated and reported to aid in understanding the relationships and to show levels of field conformity to the design parameter.

QL can only be calculated when "n" is three or larger, so in Table 1 it is not shown for processes with less than two tests. There are columns for PF's for voids properties (special for these projects) and for the usual QPM 2 elements. The actual incentive/disincentive (I/D) dollars paid for the various process elements is shown; it is the combination of the two PF's. Contractors' code is used by CDOT to identify the various HBP contractors. Grading S (SP 3/4" nominal) was used on both projects. "F" was added here to show the plotted grading curves were above the maximum density line.

As stated above, if the contractor's VT and QC tests do not compare within a probability of 0.5%, the process element PF must be based on CDOT VT's. The CDOT QL's are listed in Table 1 for comparison to the QC QL's. For both projects the weighted average CDOT QL for QPM 2 elements was 88.1, compared with the contractor's 94.1. By CDOT, the I/D\$ would have been close to zero, compared with \$64,923 by the contractor's QL. For voids properties, by CDOT, the I/D\$ would have been about \$25,000, compared with \$18,205 by the contractor's data. Contractor pay was almost \$70,000 more than if based on CDOT tests. F and t test results show the differences in means and SD's, and consequently QL, could have occurred by random chance within the probability levels stated. Examination of element probabilities shows the lowest values, for contractor QC's to VT's, was for density on process No. 2, Project 11755 (See Table 1, Figure SS-1, and Figures 4 and 5). Values were 0.064 for F and 0.018 for t, close to the 0.5% critical value, but OK.

Figure SS-1 is a copy of the spreadsheet for the above process. Compare the SD's and means. For the contractor and CDOT VT sets, the match is good (0.02 difference in SD's and 0.05 in means). The match is not as good for the QC and contractor's VT (0.19 difference in SD's and 0.35 in the means), but the probabilities are acceptable. In this density process, the contractor's verification test No. 11 was 89.7, more than 2 x V below the lower limit of 92.0. V is approximately one historical SD, and is 1.1 for

density. Any element test value more than 2 x V out of limits is made into a separate one-sample process and evaluated by a special formula. In Figure SS-1, this test has been removed from the process (also, the accompanying CDOT split, which compared favorably). The spreadsheet is not shown with the test included, but Figure 2 graphically portrays the *F* probability curve with No. 11 included (where SD difference was 0.40). At test No. 20 the probability dropped to 0.005 and at No. 24 to below 0.001. Figure 4 is a plot of the *F* probability calculations from Figure SS-1 (without No. 11) and shows the dramatic difference in line slopes with the single low, outlying value removed.

The second page of Table 1 has the processes grouped by elements, first those in regular QPM 2, and second, those associated with VA. A weighted average and total line is shown for each element. Summarized at the bottom of the page are the six elements for the two projects. Below that summary is a smaller box with information for QL, PF and I/D\$. Shown, also, is comparable information from the 1997 regular QPM 2 projects, the 1997 VA projects, and by the 1998 VA criteria. The I/D values in the Table 1 summary show that if the two QCFP projects had been evaluated under the 1998 VA formulas, a disincentive of \$15,070 rather than an incentive of \$83,127 would have been assessed. Also note that the Region 2 VA projects, by 1998 criteria, would have an incentive of \$69,720 rather than \$201,468. For the QCFP projects, the difference in pay is related to the special incentive PF's for the VMA and AV (with no negatives), while the usual elements had higher QL's than historical averages. For the Region 2 VA projects, the difference is because the pay-factor formulas used (similar to QPM 1) were more favorable to the contractor than the QPM 2 formulas used in 1998 VA specifications.

Table 2 has a more comprehensive array of data from the various type of HBP projects constructed since 1992. It includes information on the number of tests, on SD, QL, PF's by QPM 1 (or VA) and QPM 2. For QCFP, looking first at density, the SD is considerably below the QPM 2 averages, and the VA Superpave values. From Figures SS-1 and SS-5, it can be seen that CDOT's VT's also have SD's lower than typical. The QL is higher than any other density QL's above in the column. Superpave void acceptance projects are displaying a trend toward lower SD's for density (while mean values are staying about the same, or lower). It is too early to say whether the 0.61 SD value is unrealistically low. The AC content values appear reasonable in comparison to QPM 2, and better than previous VA projects. The VMA SD and QL values are in line with previous VA data. All the VMA values are high, showing the tolerance limits and job-mix targets are easy to meet. Either the tolerances should be tightened or the "W" factor decreased, or both.

Air voids element values for QCFP show low compliance with specifications. SD is higher than on the VA projects, and QL is much lower. On previous VA projects the universal target of 4.0% has proved difficult to meet. On two of the 1997 Region 2 VA projects the target was changed to 3.5%. Without this change, the average 1997 AV QL might have been lower. On the QCFP projects, after some significant problems at startup (see 11755 Process 1 and 11600 processes 1 & 2), the air voids were close to target, but the SD's were high. The air voids test is really a calculation from two test procedures; bulk specific gravity on laboratory-compacted specimens and maximum theoretical specific gravity (Rice). Much

training and practice are required for skills to be developed for these tests. CDOT laboratory results show they have developed the necessary skills. Their average air void SD on their VT's was 0.64, not far above the average of 0.56 for the SP pilot VA projects. This shows that most of the QC variability was probably related to testing rather than production. As private labs and contractors gain experience, their values are expected to fall more in line with CDOT's.

THE F-TEST and t-TEST PROCEDURES and GRAPHS of PROBABILITY DATA

Figures SS-1 through SS-8 are copies of the spreadsheets used for calculating probabilities for density, AC%, VMA and AV elements on the two projects. (Only the major processes are shown, spreadsheets for the startup processes were made by field personnel, but to avoid clutter, are not shown here). Spreadsheets were also prepared for each specified sieve on each project. All met the F and t criteria easily. Field gradations will not be in VA specifications, so no further reference will be made.

In the SS series of figures, three major sections are to the right of the test data fields, each with two columns of probability calculations. Figures 2 through 25 are graphic plots of the probability values from the spreadsheets. On the spreadsheets, the first F and t columns compare contractor VT's with CDOT's based on running 5-sample splits; the data is plotted on the figures as the medium weight line.

The second pair of F and t columns is for information only and shows trends in the accumulative VT comparisons. In the Figures, the lightest weight line represents this pair of columns. In searching for problems when the running 5-sample splits are not acceptable, the columns may be useful. For information only, the third pair of columns provides an accumulated analysis of the QC to Contractor VT comparison. On the graphs, this plot is represented by the heaviest line. At completion of the process, based on all the tests (last data entries), a decision is made whether to pay by QC or CDOT VT data. Values in the columns show trends and should warn the contractor in time to correct problems.

In the first pair of columns, for all elements on both projects, only three cases of **Alerts** (all for low t values) on VT for splits after test No. 4 are shown, (tests 3 and 4 are for information only). They were as follows:

- (1) For density on 11755 (Figure SS-1 and Figure No 5). This **Alert** at pair No. 18 appears to have been a random anomaly. The running set of five tests had very low, similar SD's. The formula predicts that the means should be nearly identical, but they were slightly different. Without corrective actions, this alert corrected itself,
- (2) For the last pair of split VT's for AC% on 11755 (Figure SS-2 and Figure 7.). No corrective action was required, as this was the last split pair, and
- (3) For air voids on 11600 (see Figure SS-7 and Figure 23). This **Alert** was for pair No. 5. Corrective actions were taken and resulted in satisfactory probabilities for the rest of the sets.

For Figures 2 through 25, paired graphs for individual elements were plotted from spreadsheet probability data. F data is on the top figure and t data on the bottom figure. The pairs are plotted from

calculations on the same sets of test values. Above the upper graph is the pertinent statistical data for each pair of graphs. On page 5, above, Figures 2 and 4 are discussed, relating to the effect of a single outlying density value in a process. By examining the block of data above the two graphs, it is apparent the outlying density test SD's affected the SD much more than the mean. In Figures 2 and 4, the two bold line plots for SD's (F) are very different, while the bold line plots for means (t) are similar (Figures 3 and 5).

REDUCING THE RATIO OF VERIFICATION TESTS TO QC TESTS

As part of this study, an experiment was done to simulate the effect on probability calculations if the number of VT's to QC's were cut in half. AC% was not evaluated because a 1:7 ratio was used and it is not expected the ratio will decrease below that. On Figures SS-1, SS-3 and SS-4 (for density, VMA and AV respectively), the VT data has been blocked off by light horizontal lines into stratas to create 1:7 ratios. This gives two to four VT's per strata. By random numbers, one was selected (heavier shading) for each strata. The other VT's were added to the QC strata and the corresponding CDOT VT values were discarded.

New spreadsheets were developed for this changed QC format and the reorganized data entered. Prints of these sheets are not included, but are in the files. Figures 6 & 7, 12 & 13, and 16 & 17 (for density, VMA, and AV, F and t calculations respectively) are plotted from the data in this experiment. In none of the three cases would the action decisions have changed had this reduced VT schedule been carried out. This suggests that the ratio of VT to QC tests can be similar to the ratio represented by this experiment without a major effect on QC acceptability decisions.

EXPERIMENT TO VERIFY CRITICAL PROBABILITY LIMITS

It has been noticed that where there are significant average differences between means or between SD's for two sets of data, the rate of probability descent is steep and rather constant. This relates to the QC to contractor's VT comparisons. As 'n' increases, the probabilities get lower and lower, though the differences in means and SD's remain about the same. The question arose whether the critical value (0.5%) for action on VT to QC comparisons should be changed as "n" increases. It was also suggested that calculating probabilities based on running sets of 10 and 20 VT's be evaluated to see if this gave a more reasonable method for acceptability decisions.

Figures 26 through 33 represent several computer-generated sets of data used to test the above questions. Mean and SD differences are as noted on each figure. In this experiment, SD, mean and "n" are the computer variables, and were purposely selected as shown. Three small graphs are included in each figure. Represented on the cumulative graph is the current spreadsheet calculation method for the QCFP projects. Running sets of 20 values (for the same groups) are shown on the next graph. The third graph shows running sets of 10 values (same groups). Figure 26 and 27, for asphalt density, compare sets of data about as different in SD's as they can be and still be acceptable (for "n" greater than 30). In Figures

28 and 29, data sets are shown with unacceptable differences after "n" equals 45 for SD's and 23 for means.

Figures 30 and 31 are for computer-generated random sets for air voids. Note these become unacceptable after 27 tests. We can say with confidence, after 27 tests, only a 0.5% probability exists that the differences in SD's and means are by random occurrence. Note that at test No. 18, the lines started upward. More values were needed to be sure of the trend. Finally, Figures 32 and 33 show plots for two sets of data related to asphalt content. The probability for the means difference becomes unacceptable at test No. 38.

The *F-test* formula for comparing SD's is independent of the means difference. Two sets of data with very different means can still have very similar variances and be acceptable for SD comparisons. The *t-test* formula includes variance and mean values for the two sets, so when comparing the means, SD has a major effect. Where the SD difference is small, only a small difference in means is allowable.

Acceptable average differences in SD's and means cannot be stated. If this were the case, average differences could be used rather than statistical calculations. Peaks and valleys in the lines are caused by the randomness of the numbers. Another set of values with the same differences would create plots with different peaks and valleys, but show similar slopes. In the computer generated groups, calculating by running sets of either 20 or 10 gave no better information for decisions than using accumulated data. The SD's for smaller sets of numbers vary in relation to the true SD for the population divided by the square root of "n" for the smaller sets. The best, most reliable calculations can be made by using all data available. We conclude from this experiment, the method of calculating the F and t probabilities and selection of critical values should remain as used in the QCFP pilots, at least for now. If the ratio of VT to QC is reduced, a distinct downward trend in probability values might become an issue only for very large tonnages (large "n"s).

CONCLUSIONS

- (1) Contractors' QC tests can be used for pay determinations with verification procedures similar to those used on the 1996 QCFP projects. For the regular QPM 2 elements, the contractor's average QL of 94.1 was much higher than the three-year average QPM 2 QL of 91.0. However, during this period, nine annual QL averages by individual contractors were more than 94.0 (for tonnages equal to or greater than on these projects). So values as high as 94.1 should be expected. CDOT's average QL of 90.0 is not statistically different from the contractor's 94.1. Both average QL's are composites of the element QL's calculated from individual sets of test values. The sets were evaluated by *F* and *t* tests. It was found the differences could have been random occurrences, within the probabilities stated.
- (2) The *F* and *t* test procedures used for these projects to compare sets of test values were workable.

 Based on differences in SD's and means, the program adequately differentiated between acceptable

and unacceptable comparisons. Errors in the spreadsheet program used in the field have been corrected and a few other minor changes made. Other, easier to use, program formats could be developed using F and t probability calculations.

- (3) VMA criteria were easily met, based on job-mix targets, resulting in a QC average QL of 94.8. This agrees favorably with values developed by CDOT on previous VA projects. On these projects, CDOT's QL was 98.1. SD's compare favorably with the VA projects. The tolerances should be tightened to ± 1.0% and the W factor reduced from 0.2 to 0.1.
- (4) AV criteria were not satisfactorily met. The average value was 4.05, only 0.05 above the target. Nevertheless, the average SD was 0.84 (compared with the VA/SP average of 0.56), resulting in a QL of 76.4 compared to the VA/SP average of 90.7 (and CDOT's VT QL of 90.5). The special PF formula did not provide enough incentive to override the production and testing problems incurred by the contractor. Because CDOT's values compare favorably with previous VA projects, we conclude that most of the contractor's problems were in testing procedures (probably in making specimens and the specific gravity tests). We believe that practice and attention to detail will solve the testing problems. Importance of this element warrants a higher W factor.
- (5) VFA averaged 72.6, within the 65-75 limit, but with a QL of only 79.8. The low QL was related to the high AV standard deviations. VMA and AV are the variables used to calculate VFA; the VMA criteria were satisfactory.
- (6) The number of verification tests in relation to the QC tests can be safely reduced, up to half, without significantly affecting decisions based on the probability values that compare sets of data.

RECOMMENDATIONS

- (1) Use the quality level analysis approach for the pilot QCFP projects planned for 1999. These projects should have as pay elements, AC%, in-place density, VMA and AV, and be designed by the SP procedure. The AV test needs particular attention. Industry will gain experience in QC voids-analysis testing on the Phase 2 VA projects planned for 1998. Based on the proficiency proved by CDOT laboratories, it is expected QC air-void testing will be acceptable for the 1999 QCFP pilots.
- (2) Decrease the W factor for VMA to 0.1. VA/SP historical average SD is 0.46. Decrease the tolerance to ± 1.0, two historical SD's for a seller's risk of 5%. Leave V at 0.6, 1.2 historical SD's for a seller's risk of 5%.
- (3) Increase the W factor for AV to 0.4. The historical VA/SP average SD is 0.56. Leave the tolerance at ± 1.2, two historical SD's for a seller's risk of 5%. Increase V to 0.7, 1.2 times the historical average SD for a seller's risk of 5%.

- (4) For the 1999 pilot QCFP projects, continue the use of the *F* and *t* test procedures to verify the contractors' QC tests. The current spreadsheet is workable and has been updated and corrected. It may be more cumbersome than necessary; so consider revisions. The most difficult parts of verifying the QC tests were the methods described in the 1997 pilot specifications for setting up the random sample selection schedules. These, along with the actual mechanics involved, need to be reviewed carefully.
- (5) Pay more attention to the VFA parameter. It is not recommended that it be a pay element.

 Nevertheless, calculate and consider it routinely when setting up the job mix formulas in the field.

 VFA is affected by the targets selected for AC%, VMA and AV.

REFERENCES

- 1. Revisions of the Standard Specifications, Sections 105, Control of Work and 106, Control of Material; to be used with the 1992 Pilot Projects, by the Staff Materials Branch, CDOT, March 1992. (OPM 1)
- 2. Revision of Sections 105 and 106, Quality of Hot Bituminous Pavement, April 25, 1995 (Reissued with minor editorial changes, March 7, 1996). CDOT, 4201 East Arkansas Avenue, Denver, CO 80222. (QPM 2)
- 3. HBP QA/QC Pilot Projects Construction in 1992, Interim Report. Report No. CDOT-DTD-R-93-14, by Bud A. Brakey, Colorado Department of Transportation, 4201 East Arkansas Avenue, Denver, CO 80222.
- 4. HBP QA/QC Pilot Projects Construction in 1993, Second Interim Report, by Bud A. Brakey, Colorado Department of Transportation, 4201 East Arkansas Avenue, Denver, CO 80222.
- 5. Hot Bituminous Pavement QC/QA Projects Constructed in 1994 and Summary of the 1992-1994 QC/QA Pilot Program, Final Report, June 1995, by Bud A. Brakey.
- 6. HBP QC&QA Projects Constructed in 1995 Under QPM 1 and QPM 2 Specifications, (1996 fourth annual report by Bud A. Brakey, Colorado Department of Transportation, 4201 East Arkansas Avenue, Denver, CO 80222.), Report No. CDOT-R-96-9.
- 7. HBP QC&QA Projects Constructed in 1996 Under QPM 2 Specifications, (1997 fifth annual report by Bud A. Brakey, Colorado Department of Transportation, 4201 East Arkansas Avenue, Denver, CO 80222.), Report No. CDOT-R-97-9.
- 8. HBP QC&QA Projects Constructed in 1997 Under QPM 2 Specifications, (1998 sixth annual report by Bud A. Brakey, Colorado Department of Transportation, 4201 East Arkansas Avenue, Denver, CO 80222.), Report No. CDOT-R-97-4.
- 9. "SuperpaveTM Level 1 Mix Design," Asphalt Institute SuperpaveTM Series No.2 (SP-2), Asphalt Institute, P.O. Box 14052, Lexington, KY 40512-4052.
- 10. HBP Pilot Void Acceptance Projects Completed in 1993-1996, (Interim report by Bud A. Brakey, Colorado Department of Transportation, 4201 East Arkansas Avenue, Denver, CO 80222.) Report No. CDOT-DTD-R-97-8.
- 11. HBP Pilot Void Acceptance Projects in Region 2 in 1997, (Interim report by Bud A. Brakey, Colorado Department of Transportation, 4201 East Arkansas Avenue, Denver, CO 80222.) Report No. CDOT-DTD-R-98-2.
- 12. From Corel Quattro Pro 7, 1996 (Spreadsheet Program), @FTEST and @TTEST functions.
- 13. Colorado Procedure 71-94 For Determining Quality Level (Percent Within Tolerance limits), 1997 Field Materials Manual. Colorado Department of Transportation, 4201 East Arkansas, Denver CO 80222.

QC/QA HOT BITUMINOUS PAVEMENT USING CONTRACTOR QC FOR PAY (Includes Voids Accept) DETAILS AND SUMMARY BY PROJECT AND ELEMENT FOR 1997 PILOT PROJECTS Table 1

	DETAILS AND SUMM	Y A		Ĭ	0		L DECN		ׅ֡֝֝֝֝֝֝֝֝֝֝֝֝֝֡֝֝֝֡֝֝֡֝֝֡֝֡֝֡֝֡֝֡֝֡֝֜֜֝֡֡֝֡֡֝֡֡֝֡֜֝֡֡֡֝֡֡֡		AND ELEMEN I FOR	"	, PILOI	ב כ	PROJECTS	<u>ئ</u>				
PROJECT AC LOCATION GRAD	SUBAC	ည္ရ လူ	MENT	BID \$	TONS 1000	TEST F	PROCESS ELEMENT DATA & CALCULATI TARG TOL+ MEAN T-M SD OL	S ELEN	MENT DA	TA & C	SD	_	Vds OPM2		ACTUAL	CNT	AGG	CDOT	QC to VER	VER V
			100000			888	Gra	dation Da	┨╬ ┡	اہا	-		7‱	-			3	일		3 -
IM 025 - 302																				
	11755	-	AC%	\$42.00	0.8	2		0.30	-	0.01				1.000	0\$	₹	S(F)			
25, Hampden South 76-28 125, Hampden South 76-28	11755		Dens% Grad	\$42.00	0.8	- 2		5.00	-	- 0.80 -1.00					0\$ 0\$	<u> </u>	S(F)			
	11755	-	Voids	\$42.00	0.8	7	4.00	1.20	2.30	-1 70			1,000	1.000	\$0	Σ	S(F)			
	11755	-	VMA	\$42.00	9.0	7		1.20		-1.05					. ₀	<u> </u>	S(F)			
For information only 76-28	11755		VFA	\$42.00	0.8	2	70.00	7.00	82.90	12.90						¥	S(F)			
:	11755		AC%	\$42.00	26.9	29	_	0.30		_	_	98.2	_	1.055	\$18,612	₹	S(F)	} -	0.863	0.232
	11755	7	Dens%	\$42	56.9	61		2.00				91.7		1.017	\$9,160	ž	S(F)		0.064	0.018
***************************************	11755	7	Grad	\$42.00	26.9	8	45.00	5.00	45.40	0.40	-8	93.9		1 038	\$8,498	Σ	S(F)	100.0	0.116	0.817
	11755	7	Voids	\$47	26.9	83	4.00	1.20				76.5	_	0.913	\$0	₹	S(F)	95.2	0.166	0.679
125, Hampden South 76-28	11755	7 7	VMA V	\$42.00	26.9	8 8	14.50	1.20	14.55			98.2	1.017	1.055	\$9,707	₹ :	S(F)	_	6	0.159
LOT INOTIMATION ONLY	01/10	7	¥->	\$42.00	6 8	8	900	 8	(3.23	323	4.90	99/				Σ	S(F)	86.7	ð	ð
PROJECT TOTALS & AVERAGES PROJECT TOTALS & AVERAGES	QPM 2		ITEM	\$42.00	27.7							94.1	5	1.032	\$36,270			86.2	•	
מוסבוסר מיסבוסר מיסב מיסב מיסבוסר מיסבוסר מיסבוסר מיסב מיסב מיסבוסר מיסבוסר מי	doc sp.	000000	1	942.00	1.17							67.3	1.008	0.984	,0/\g			97.6		
															\$45,977					
NH 0021-022																				
Colorado Bvd 64-22	11600	-	AC%	\$36.00	2.1	2	_	0:30	5.23	0.23	0.20	62.8		0.919	(\$1,878)	Σ	S(F)	78.5		
	11600	-	Dens%	\$36.00	2.1	4		2.00	93.75			100.0		1.030	\$1,159	Σ	S(F)	¥		
	11600		Grad	\$36.00	2.1	2	45.00	2.00		-1.40	3.80	62.9		0.920	(\$1,243)	₹	S(F)	Ą		
	11600	-	Voids	\$36.00	2.1	ς.		1.20	_		0.36	0.0	-	0.500	\$0	조	S(F)	0.0		
-	11600	_	AMA !	\$36.00	2.1	2		1.20	12.80		0.37	0.0	1.000	0.500	\$0	조	S(F)	67.4		
For information only 64-22	11600	-	VFA	\$36.00	21	2	20.00	- 200 2	98.10	18.10	5.03	8.9				₹	S(F)	0.0		
•	11600	2	AC%	\$36.00	6.0	-	2.00	0:30	: .	-0.12				1.000	\$0	₹	S(F)			
		7	Dens%	\$36.00	0.9	7	94.00	2.00	95.30	6.1				1.000	\$0	조	S(F)			
	8	2	Grad	\$36.00	60	-	37.00	200		200				1.000	\$0	ž	S(F)			
	11600	5	Voids	\$36.00	6.0	-	4.00	1.20		-2.30			1.000	0.500	\$0	ž	S(F)			
	11600	7 (AMY :	\$36.00	6.0		15.50	1.20	11.80	-3.70			1.000	0.500	\$0	∑ :	S(F)			
	200	,	2	00.00	<u> </u>	- 	300	300	0000	2 2 2						Z	(H)S			
	11600	ကျ	AC%	\$36.00	24.5	49	4.80	0.30	4.80			8.06		1.010	\$2,549	조	S(F)	75.6	0.071	0.338
Colorado Bvd 64-22	1600	n 6	%suaO	7 6	24.5	24 69	94.00	8 8	94.35			6 9			\$24,232	<u> </u>	S(F)			0.756
Co Pod			71.77							200			*	77.		200	7	*		5
	14600	, c	Spio	9 6	2.4.0	ž ć	3 5	2 5	DC. 4			82.8			9	Z 3	S(F)			0.736
ajuo u	11600	2 (V V	838.00	0.4.0		5.5	3 5	06.90				<u> </u>	<u> </u>	36,49 8	2 3	(<u>F</u>)		0.281	0.255
	3	, 🖁		3		?	800	3	8	3	***	 ? R				2	(T)	5 8	5	ź
PROJECT TOTALS & AVERAGES	QPM 2		ITEM	\$36.00	27.5							94.1		1.031	\$28,653			90.0		
PROJECT TOTALS & AVERAGES	Vds Acp		ITEM	\$36.00	27.5							81.2	1.008	0.946	\$8,498			91.1		
															\$37,150					
TOTALS & AVERAGES (2 Projects) Regular Elements	Regular Elem	ents		\$39.01	55.2							00000		1.032	\$64,923			88.1		
IOIALS & AVERAGES (2 Projects) Voids & VMA	Voids & VMA			\$39.01	55.2								1.00887	996.0	\$18,205			94.5		
				ł						اد	GRAND TOTAL	₹			\$83,127					
						-	. Would ha	ve been 0	.999, exce _l	pt special	provision	did not a	llow disine	entive for	Would have been 0.999, except special provision did not allow disincentive for volumetric properties	roperties				!

QC/QA HOT BITUMINOUS PAVEMENT USING CONTRACTOR QC FOR PAY (Includes Voids Accept) DETAILS AND SUMMARY BY PROJECT AND ELEMENT FOR 1997 PILOT PROJECTS Table 1

	-1	I AIL	AILS AND		¥Γ	Y BY	. {!	IJb		AND ELEMEN		J S	- I	7 PILO	OT P	PROJECTS	ŀ				
	GRAD N	SUBAC	2 <u>8</u>	MENT	SID \$	1000	 מייי		TARG TOL+ - MEAN	⊼⊢		SD	¥ 0	Vds TOPM2		ACTUAL	T L	AGG C	CDOT Q	QC to VER	# 5
	<u>დ</u>						3000		Gradation	12	#8 Sieve	₩) }			d		<u>.</u>
		PROJECT DATA	ECT [JATA S	SORTED	AND	SUM	MARIZ	SUMMARIZED BY	ELEMENT	NT]
	76-28	11755	-	AC%	\$42.00	8.0	2	4.90	0.30	4.91	0.01				1.000	eş S	ξ	S(F)			
n South		11755	7	AC%	\$42.00	26.9	29	4.80	0.30	4.76	-0.04		98.2		1.055	\$18,612	₹	S(F)	•	0.863 0.	0.232
Colorado Bvd	64-22	11600	- ۲	% % \$ 0	\$36.00	2.1	ი -	00.6	0.30	5.23	0.23	0.20	62.8		0.919	(\$1,878)	Σ <u>3</u>	S(F)	78.5		
		11600	4 W	AC%	\$36.00	24.5	- 64	4.80	0.30	8 4	0.00	0.18	808		1010	30 \$2.549	2 2	S(T) S(E)	756 0	0 071	0 338
QC for Pay Weighted Averages & Totals for AC%	Totals (or AC%				55.2	116	4.81	0.30	4.80	0.01	0.15	93.4		1.028	\$19,283			╁	1	
I 25, Hampden South 76-	76-28	11755	-	Dens%		9.0	-	94.00	2 00	93.20	-0.80	-			1.000	S	Σ —	S(F)	_	_	
n South		11755	2	Dens%	\$42.00	26.9	5	94.00	2.00	92.70	-1.31		91.7		1.017	\$9,160	조	S(F)	73.2 0	0.064 0.	0.018
		11600	- 1	Dens%	\$36.00	2.1	4	94.00	2.00	93.75	-0.25	0.88	100.0		1.030	\$1,159	조	S(F)	¥ Z		
Colorado Bvd 64-	64-22	11600	~ ~	Dens%	\$36.00	0.9	2 8	94.00	2.00	95.30	8 8	, ,	6		1.000	\$0	Z 3	S(F)	980		Ş
einhted Average	Totals	or Densih		2000	20.00	55.2	111	8 8	3 5	93.53	9 0	+	93.0		200	324,232	2		+	0000	0./35
5 0000			2	***************************************	**************************************	7.00	- - -	300	30000	93.32		-8			1.034	334,551			85.4	_	33333
		11755	-	Grad	\$42.00	0.8	7	45.00	2.00	44.00	-1.00	800 800			1.000	\$0	Σ	S(F)			
n South	76-28	11755	7	Grad	\$42.00	26.9	35	45.00	5.00	45.40	0.40		93.9		1.038	\$8,498	ž	S(F)	0	0.116 0.	817
Colorado Byd		11600	- ‹	Grad	\$36.00	1.2	۰ ۵	45.00	8 8	43.60	9.1.	3.80	62.9		0.920	(\$1,243)	¥ :	S(F)	₹		
		168	3 6	Grad	\$36.00	24.5	27	45.00	8 9	45.10	3. O	2.60	91.5		1.000	\$0 \$3.833	Z Z	S(F)	Ą	ž	č
QC for Pay Weighted Averages & Totals for Gradation	Totals 1	or Gradal	ioi			55.2	29	44.87	5.00	45.12	0.25	╀╌	91.5		1.025	\$11,088			╁	╁	
25. Hampden South 76-	76-28	11755	-	1 VMA \$42	842 00	8	,	14.50	1 20	13.45	-1.05			- -	1 000		3	(1/0			
		11755	٠ ،	VMA	\$42.00	26.9		14.50	2 5	14.55		25	98,7	1.000	1 055	\$0.707	2 3	((700		, Ç
		11600	-	VMA	\$36.00	2.1		15.50	1.20	12.80	-2.70		0.0		0.500	30,'66	2 2	S(F)		U.323 U	
Colorado Bvd 64-	64-22	11600	7	VMA	\$36.00	6.0		15.50	1.20	11.80	-3.70		641.0 660.0 660.0		0.500	S S	Σ	S(F)			
Colorado Bvd 64	64-22	11600	3	VMA	\$36.00	24.5	49	14.50	1.20	14.90	0.40	0.33	99.5	1.019	1.055	\$8,498	Υ	S(F)	100 0	0.281 0	0.255
QC for Pay Weighted Averages & Totats for Void in Mineral Agg	Totals	for Void in	Minera	l Agg		55.2	8	14.55	1.20	14.58	0.02	0.43	94.8	1.017	1.024	\$18,205			98.1		
1 25, Hampden South 76.	76-28	11755	-	Voids	\$42.00	0.8		4.00	1.20	2.30	-1.70			1.000	1.000	\$0	7	S(F)		_	
1 South	76-28	11755	7	Voids	\$42.00	26.9	33	4.00	1.20	3.96	-0.04	1.01	76.5		0.913	\$0	잪	S(F)	95.2 0	0.166 0.	0.679
	64-22	11600	- 1	Voids	\$36.00	2.1	. S	4.00	1.20	1.58	-2.42	0.36	0.0		0.500	\$0	조	S(F)	0.0		
Colorado Bvd 64	64-22	11600	2 6	Voids	\$36.00	0.9	- ę	8.8	1.20	1.70	-2.30	Ç.	•		0.500	င္တ မ	<u> </u>	S(F)	344	7	Ş
eighted Average	Totals	or Air Voi	2 %	solos	\$30.00	55.2	9 8	3 8	1 20	4.30	20.00	+	87.8	000.	0.948	2 8	Z	(<u>f</u>)	96.6	0.983	0.736
	<u> </u>			***************************************		1000000	3000		37.	300	3	-80		-	9	9	-		C.O.		***
VFA not specified, for info only		11755	۰ -	VFA	\$42.00	9.0	3 2	70.00	7.00	82.90	12.90	335 335	,		0.814		Σ <u>3</u>	S(F)	330 330		
VFA not specified, for info only		11600	,	VFA	\$36.00	2.1	3 40	20.00	2 00	88.10	18 10	5.03	0.0		2 60		2 2	() ()) o	5	<u> </u>
VFA not specified, for info only		11600	7	VFA	\$36.00	6.0	-	70.00	7.00	85.60	15.60				0690		2	S(F)	3		
VFA not specified, for info only	\dashv	11600	6	VFA	\$36.00	24.5	6	70.00	7.00	69.80	-0.20	4.31	90.3		1,006		Σ	S(F)	86.9	š	ð
QC for Pay Weighted Averages & Totals for % Voids Filled/Asphall	Totals	for % Voic	ts Filled	/Asphalt		55.2	8	70.00	7.00	72.62	2.62	4.63	8.62		0.934				81.8		
				AC%			116	4.81	\vdash	4.80	\vdash	Н	33.4	•	1.028	\$19,283				orania de la composición dela composición de la composición de la composición dela composición dela composición dela composición de la composición dela	0000000
				Dens%			7	94.00	-	93.52	-	-	95.5	•	1.034	\$34,551					
				Grad			3	44.87 14 55	5.00	45.12	0.25	2.35	91.5	1,70	1.025	\$11,088					
				Voids			8	408	-	4 05	-	+	+	۲.	1.024	\$18,203					
				VFA (for Info C	r Info Only)	S S	8	70.00	7.00	72.62	2.62	4.63	500	3000	0.934	3					
QC for Pay: AC%, Density & Gradation	y & G	radation			\$39.01	55.2							94.1		⊢	\$64,923					
QC for Pay: VMA and AV													П	1.0089	Н	\$18,205					
If by Proposed: 1998 VA Elements: Density, AC%, Voids &	Eleme	nts: De	nsity.	AC%, Vc	oids & VMA	∡							89.4		-	(\$15,070)					
1997 QPM 2 (Regular QC/QA)	8				\$31.26	378.9						-1			1.018	\$65,710					
1997 Voids Acceptance Projects in Region 2	rojec	ts in Re	gion		\$31.83	273.8							91.4	1.021	1.008	\$201,468 By proposed 1998 VA spec, \$69,720.	Ву ргоро	sed 1998	VA spec,	69,720.	

Table 2
Density, Asphalt Content, VMA and AV Test Data
Void Acceptance Compared to QPM 1 & 2 and QC for Pay Projects

		"Numbe	"n" or Number of tests		-	Standard	Standard Deviation	и		Quality Level	Level			QPM 1 PF or VA PF	QPM 1 PF or VA PF			QPM 2 PF	2 PF	
Group Identification	Dn	AC	VMA	AV	Du	AC	VMA	AV	Dn	AC	VMA	AV	Dn	AC	VMA	AV	Ω	AC	VMA	AV
VA, Texas Gyr. Design Constructed in 1993-96	615	918 316	316		316 1.00	61.0	0.36	0.51	84.1	86.3	93.4	92.9	0.978 1.000	1.000	1.023	1.024	996'0	766.0	1.022	1.024
1991-95, QPM 1	5729	5729 3092			1.01	0.15			88.1	90,4			1.017 1.030	1.030			0.992 1.017	1.017		
VA, Superpave, 1996	171	98	98	98	0.87	0.17	0.49	0.58	7.77	9:62	91.2	82.6	0.892	0.956	1.002	0.978 0.907	0.907	0.944	1.013	096.0
VA, Superpave, 1997	548	275	275	275	0.81	0.20	0.45	0.55	7:06	81.2	95.1	93.2	1.012	0.994	1.031	1.027	0.984	726.0	1.028	1.029
1995-97, QPM 2	2785	1579			0.93	0.16			92.3	90.1			NA	NA			1.017	1.009		
QC for Pay, 1997	117	116	90	90	90 0.61	0.15	0.43	0.84	95.5	93.4	94.8	76.3			1.017	1.000	1.034	1.028	1.024	0.907

Contractor QC vs Verif

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QC Data Verification Program

Version 1.004

PROGRAM FOR COMPARING CONTRACTOR QUALITY CONTROL FOR PAY & CDOT VERIFICATION DATA SETS

PROJECT: IM0252-302 Subaccount 11755

Location: Begin Date: 1 25, Hampden - South 8/3/97

In-Place Density

Enter Data in Yellow Blocks Only

Target	94.0				Note: F-	test compares diffe	rences in SD's, t-test differen	es in means.
			Contract	CDOT		Probabilities	Probability, Cumulative	Probabilities
Contra	ctor QC	CONTR. COM	Verif	Verif	•	F & t-test	F & t-test	F & t-test
Mean	92.75	92.56	92.40	92.35	Warnin	0.05	0.05	0.05
SD	0.35	0.49	0.54	0.56	Alert	0.01	0.005	0.005
"n"	28	66	38	32				

Col 1										73.22	38 77.06	87.09		98.76	QL
No. Contractor's Independent R Split Verif Tests Contr. CDOT F-test t-test Status F-test T-Test T-Test T-Test T-Test Status F-test T-Test	15	14	13	12	11	10	9	8	7	6	5	4	3	2	
No. QC Results Contr. CDOT F-test 1-test Status F-test T-Test Status T-Test Test T-Test T-Test Test T-Test Test T-Test Test T-Test Test T-Test Test T-Test Test	or Final)	itive (For	Accumu	ication	, on Verifi	For Info	rification	on on Ve	For Actio					1.0283	
1	vs Verif	tor QC vs	Contract	ation	tive Verific	Cumulat	5 Sample	Sets of	Running	if. Tests	Split Ver	ependent R	or's Ind	Contract	Strata
2 91.4 91.1 92.0 91.9 0.741 0.270 OK 0.741 0.270 OK 0.753 OK 91.9 92.6 0.446 0.753 OK 0.741 0.270 OK 0.753 OK 0.446 0.753 OK 0.771 0.795 OK 0.739 0.788 OK 92.9 92.7 92.1 0.564 0.643 OK 0.920 0.475 OK 0.928 0.379 OK 0.130 0.00 OK 0.928 0.379 OK 0.338 0.00 OK 0.339 0.340 OK 0.332 0.00 OK 0.339 0.340 OK 0.3357 0.00 OK 0.339 0.340 OK 0.3357 0.00 OK 0.339 0.340 OK 0.3357 0.00 OK 0.335 0.340 OK 0.335 0.357 0.00 OK 0	t Status	T-Test	F-test	Status	T-Test	F-test	Status	t-test	F-test	CDOT	Contr.	ults	C Res		No.
3 92.0 91.9 0.741 0.270 OK 0.753 OK 0.446 0.753 OK 0.753 OK 0.753 OK 0.753 OK 0.771 0.795 OK 0.788 OK 0.789 OK 0.920 0.475 OK 0.787 OK 0.920 0.475 OK 0.780 0.724 0.211 OK 0.938 0.3379 OK 0.130 0.00 9 93.5 93.2 92.7 0.398 0.034 Warm 0.524 0.211 OK 0.338 0.00 <					i	l]	92.4	92.4				1
4 91.9 92.6 0.446 0.753 QK 0.753 QK 0.753 QK 0.771 0.795 QK 0.771 0.795 QK 0.771 0.795 QK 0.739 0.788 QK 0.774 0.795 QK 0.739 0.788 QK 0.783 QK 0.783 QK 0.783 QK 0.785 QK 0.783 QK 0.780 QK 0.872 QK 0.130 0.00 10 92.5 93.2 92.7 0.398 0.034 Warm 0.587 0.120 QK 0.412 0.00		1			l					91.1	91.4				2
5 92.7 92.1 0.771 0.795 OK 0.771 0.795 OK O.771 0.795 OK 0.771 0.795 OK 0.773 0.795 OK 0.739 0.788 OK OK 0.788 OK OK 0.788 OK 0.788 OK 0.788 OK 0.788 OK 0.920 0.475 OK 0.788 OK 0.920 0.475 OK 0.788 OK 0.920 0.475 OK 0.130 0.00 9 93.5 93.2 92.2 0.445 0.034 Waim 0.527 0.121 OK 0.412 0.00 11 92.4 92.2 92.0 <t< td=""><td></td><td>Ī</td><td></td><td>oĸ</td><td>0.270</td><td>0.741</td><td>ок</td><td>0.270</td><td>0.741</td><td>91.9</td><td>92.0</td><td></td><td></td><td></td><td>3</td></t<>		Ī		oĸ	0.270	0.741	ок	0.270	0.741	91.9	92.0				3
6 93.1 92.4 92.4 0.771 0.795 0K 0.739 0.788 0K 7 92.9 92.7 92.1 0.564 0.643 0K 0.920 0.475 0K 8 92.9 92.4 92.2 0.445 0.591 0K 0.928 0.379 0K 0.130 0.00 9 93.5 93.2 92.6 0.445 0.034 Warn 0.724 0.211 0K 0.338 0.00 10 92.5 93.2 92.7 0.398 0.034 Warn 0.587 0.120 0K 0.412 0.00 11 92.4 92.0 92.0 0.345 0.034 Warn 0.528 0.120 0K 0.506 0.00 12 92.2 92.0 92.3 0.178 0.290 0K 0.467 0.196 0K 0.687 0.00 13 92.6 92.0 91.7 0.397 0.255 0K 0.522 0.138 0K 0.576 0.00 14 92.7 92.2 92.4 0.566 0.710 0K 0.525 0.185 0K 0.492 0.00 15 92.6 92.0 92.2 0.050 0.495 Warn 0.476 0.240 0K 0.412 0.00 16 92.7 91.8 92.1 0.238 0.280 0K 0.396 0.374 0K 0.302 0.00 17 92.3 92.3 92.4 0.688 0.295 0K 0.396 0.374 0K 0.302 0.00 18 92.7 82.7 92.9 0.858 0.003 Alert 0.550 0.451 0K 0.275 0.00 19 92.4 92.4 92.0 0.976 0.554 0K 0.558 0.327 0K 0.279 0.00 20 92.7 93.1 93.6 0.560 0.404 0K 0.896 0.523 0K 0.151 0.00 21 92.6 91.8 91.5 0.339 0.910 0K 0.896 0.523 0K 0.151 0.00 0.00 0.00 0.00 0.00 0.491 0K 0.896 0.523 0K 0.151 0.00 0.00 0.00 0.00 0.00 0.491 0K 0.896 0.523 0K 0.151 0.00 0.00 0.00 0.00 0.00 0.491 0K 0.596 0.593 0K 0.279 0.00 0.00 0.00 0.00 0.00 0.00 0.00 0				ОК	0.753	0.446	ОK	0.753	0.446	92.6	91.9				4
7 92.9 92.7 92.1 0.564 0.643 0K 0.920 0.475 0K 92.9 92.4 92.2 0.445 0.591 0K 0.928 0.379 0K 0.130 0.00 9 93.5 93.2 92.6 0.445 0.034 Warn 0.724 0.211 0K 0.338 0.00 10 92.5 93.2 92.7 0.398 0.034 Warn 0.587 0.120 0K 0.412 0.00 11 92.4 82.0 92.0 0.345 0.034 Warn 0.587 0.120 0K 0.506 0.00 12 92.2 92.0 92.3 0.178 0.290 0K 0.467 0.196 0K 0.687 0.00 12 92.2 92.0 91.7 0.397 0.255 0K 0.522 0.138 0K 0.576 0.00 14 92.7 92.2 92.4 0.566 0.710 0K 0.525 0.185 0K 0.576 0.00 15 92.8 92.0 92.2 0.050 0.495 0.495 0.185 0K 0.492 0.00 16 92.7 91.8 92.1 0.238 0.280 0K 0.390 0.340 0K 0.412 0.00 17 92.3 92.3 92.4 0.688 0.295 0K 0.390 0.340 0K 0.302 0.00 17 92.3 92.3 92.4 0.688 0.295 0K 0.396 0.374 0K 0.357 0.00 18 92.4 92.4 92.0 0.976 0.554 0K 0.558 0.327 0K 0.275 0.00 19 92.7 93.1 93.6 0.560 0.404 0K 0.896 0.523 0K 0.279 0.00 19 92.7 93.1 93.6 0.560 0.404 0K 0.896 0.523 0K 0.151 0.00 121 92.6 91.8 91.5 0.339 0.910 0K 0.795 0.421 0K 0.104 0.00 0K				OK	0.795	0.771	OΚ	0.795	0.771	92.1	92.7				5
8 92.9 92.4 92.2 0.445 0.591 0K 0.928 0.379 0K 0.130 0.00 9 93.5 93.2 92.8 0.445 0.034 Warn 0.724 0.211 0K 0.338 0.00 10 92.5 93.2 92.7 0.398 0.034 Warn 0.587 0.120 0K 0.412 0.00 11 92.4 \$2.0 92.0 0.345 0.034 Warn 0.528 0.120 0K 0.506 0.00 12 92.2 92.0 92.3 0.178 0.290 0K 0.467 0.196 0K 0.687 0.08 13 92.6 92.0 91.7 0.397 0.255 0K 0.522 0.138 0K 0.576 0.00 14 92.7 92.2 92.4 0.566 0.710 0K 0.525 0.185 0K 0.492 0.00 15 92.5 92.0 92.2 0.050 0.495 Warn 0.476 0.240 0K 0.412 0.00 16 92.7 92.3 92.1 0.238 0.280 0K 0.390 0.340 0K 0.302 0.00 17 92.3 92.3 92.4 0.688 0.295 0K 0.390 0.340 0K 0.357 0.00 18 92.7 \$2.7 \$2.9 0.858 0.003 Alert 0.550 0.451 0K 0.275 0.00 18 92.7 92.4 92.0 0.976 0.554 0K 0.558 0.327 0K 0.279 0.00 20 92.7 93.1 93.6 0.560 0.404 0K 0.896 0.523 0K 0.151 0.00 21 92.6 91.8 91.5 0.339 0.910 0K 0.795 0.421 0K 0.104 0.00		l		ОK	0.788	0.739	ок	0.795	0.771	92.4	92.4	93.1			6
9 93.5 93.2 92.6 0.445 0.034 Warn 0.724 0.211 CK 0.338 0.00 10 92.5 93.2 92.7 0.398 0.034 Warn 0.587 0.120 CK 0.412 0.00 11 92.4 92.0 92.0 0.345 0.034 Warn 0.528 0.120 CK 0.506 0.00 12 92.2 92.0 92.3 0.178 0.290 CK 0.467 0.196 CK 0.687 0.08 13 92.6 92.0 91.7 0.397 0.255 CK 0.522 0.138 CK 0.576 0.00 14 92.7 92.2 92.4 0.566 0.710 CK 0.525 0.185 CK 0.492 0.00 15 92.6 92.0 92.2 0.050 0.495 Warn 0.476 0.240 CK 0.412 0.00 16 92.7 91.8 92.1 0.238 0.280 CK 0.390 0.340 CK 0.302 0.00 17 92.3 92.3 92.4 0.688 0.295 CK 0.396 0.374 CK 0.357 0.00 18 92.7 82.7 92.9 0.858 0.003 Alert 0.550 0.451 CK 0.275 0.00 19 92.4 92.4 92.0 0.976 0.554 CK 0.596 0.523 CK 0.279 0.00 20 92.7 93.1 93.6 0.560 0.404 CK 0.896 0.523 CK 0.151 0.00 21 92.6 91.8 91.5 0.339 0.910 CK 0.795 0.421 CK 0.104 0.00		1		οκ	0.475	0.920	οĸ	0.643	0.564	92,1	92.7	92.9			7
10 92.5 93.2 92.7 0.398 0.034 Warm 0.587 0.120 OK 0.412 0.00 11 92.4 \$2.0 92.0 0.345 0.034 Warm 0.528 0.120 OK 0.506 0.00 12 92.2 92.0 92.3 0.178 0.290 OK 0.467 0.196 OK 0.687 0.06 13 92.6 92.0 91.7 0.397 0.255 OK 0.522 0.138 OK 0.576 0.06 14 92.7 92.2 92.4 0.566 0.710 OK 0.525 0.185 OK 0.492 0.0 15 92.6 92.0 92.2 0.050 0.495 Warm 0.476 0.240 OK 0.412 0.00 16 92.7 91.8 92.1 0.238 0.280 OK 0.390 0.340 OK 0.302 0.00 17 92.3 92.3 92.4 0.688 0.295 OK 0.390 0.340 OK 0.357 0.00 18 92.7 82.7 92.9 0.858 0.003 Alert 0.550 0.451 OK 0.275 0.00 19 92.4 92.4 92.0 0.976 0.554 OK 0.558 0.327 OK 0.279 0.00 20 92.7 93.1 93.6 0.560 0.404 OK 0.896 0.523 OK 0.151 0.00 21 92.6 91.8 91.5 0.339 0.910 OK 0.795 0.421 OK 0.104 0.00	3 Warm	0.023	0.130	OK	0.379	0.928	οĸ	0.591	0.445	92.2	92.4	92.9			8
11 92.4 \$2.0 92.0 0.345 0.034 Warm 0.528 0.120 OK 0.506 0.00 12 92.2 92.0 92.3 0.178 0.290 OK 0.467 0.196 OK 0.687 0.00 13 92.6 92.0 91.7 0.397 0.255 OK 0.522 0.138 OK 0.576 0.00 14 92.7 92.2 92.4 0.566 0.710 OK 0.525 0.185 OK 0.492 0.0 15 92.6 92.0 92.2 0.050 0.495 Warm 0.476 0.240 OK 0.412 0.00 16 92.7 91.8 92.1 0.238 0.280 OK 0.390 0.340 OK 0.302 0.00 17 92.3 92.3 92.9 0.688 0.295 OK 0.396 0.374 OK 0.357 0.00 18 92.7 82.7	2 Warm	0.022	0.338	OK	0.211	0.724	Warn	0.034	0.445	92.6	93.2	93.5			9
12 92.2 92.0 92.3 0.178 0.290	200000000000000000000000000000000000000	0.071	1	OK	0.120	0.587	Warn	0.034	0.398	92.7	93.2	92.5			10
13	4 O K	0.074	0.506	OK	0.120	0.528	Warn	0.034	0.345	92.0	92.0	92.4			11
14 92.7 92.2 92.4 0.566 0.710 OK 0.525 0.185 OK 0.492 0.00 15 92.6 92.0 92.2 0.050 0.495 Warn 0.476 0.240 OK 0.412 0.00 16 92.7 93.8 92.1 0.238 0.280 OK 0.390 0.340 OK 0.302 0.00 17 92.3 92.3 92.4 0.688 0.295 OK 0.396 0.374 OK 0.357 0.00 18 92.7 82.7 92.9 0.858 0.003 Alert 0.550 0.451 OK 0.275 0.00 19 92.4 92.4 92.0 0.976 0.554 OK 0.558 0.327 OK 0.279 0.00 20 92.7 93.1 93.6 0.560 0.404 OK 0.896 0.523 OK 0.151 0.02 21 92.6 91.8 91.5 0.339 0.910 OK 0.795 0.421 OK 0.104 0.00	P0000000000000000	0.095	4	ОK	0.196	0.467	ок	0.290	0.178	92.3	92.0	92.2			12
15 92.6 92.0 92.2 0.050 0.495 Warr 0.476 0.240 OK 0.412 0.00 16 92.7 93.8 92.1 0.238 0.280 OK 0.390 0.340 OK 0.302 0.00 17 92.3 92.3 92.4 0.688 0.295 OK 0.396 0.374 CK 0.357 0.00 18 92.7 62.7 92.9 0.858 0.003 Alert 0.550 0.451 CK 0.275 0.00 19 92.4 92.4 92.0 0.976 0.554 OK 0.558 0.327 OK 0.279 0.00 20 92.7 93.1 93.6 0.560 0.404 OK 0.896 0.523 OK 0.151 0.00 21 92.6 91.8 91.5 0.339 0.910 OK 0.795 0.421 OK 0.104 0.00	4 OK	0.064	0.576	OK	0.138	0.522	OΚ	0.255	0.397	91.7	92.0	92.6			13
16 92.7 93.8 92.1 0.238 0.280 0K 0.390 0.340 0K 0.302 0.00 17 92.3 92.3 92.4 0.688 0.295 0K 0.396 0.374 0K 0.357 0.00 18 92.7 82.7 92.9 0.858 0.003 Alert 0.550 0.451 0K 0.275 0.00 19 92.4 92.4 92.0 0.976 0.554 0K 0.558 0.327 0K 0.279 0.00 20 92.7 93.1 93.6 0.560 0.404 0K 0.896 0.523 0K 0.151 0.00 21 92.6 91.8 91.5 0.339 0.910 0K 0.795 0.421 0K 0.104 0.00	200000000000000000000000000000000000000	0.040	0.492	ОК	0.185	0.525	OK	0.710	0.566	**********	92.2	92.7			14
17 92.3 92.4 0.688 0.295 0K 0.396 0.374 CK 0.357 0.00 18 92.7 82.7 92.9 0.858 0.003 Alert 0.550 0.451 CK 0.275 0.00 19 92.4 92.4 92.0 0.976 0.554 OK 0.558 0.327 CK 0.279 0.00 20 92.7 93.1 93.6 0.560 0.404 OK 0.896 0.523 CK 0.151 0.00 21 92.6 91.8 91.5 0.339 0.910 CK 0.795 0.421 CK 0.104 0.00	5 Warn	0.025	0.412	OK	0.240	0.476	Warn	0.495	0.050	92.2	92.0	92.6			15
18 92.7 82.7 92.9 0.858 0.003 Alert O.550 0.451 OK 0.275 0.07 19 92.4 92.4 92.0 0.976 0.554 OK 0.558 0.327 OK 0.279 0.07 20 92.7 93.1 93.6 0.560 0.404 OK 0.896 0.523 OK 0.151 0.02 21 92.6 91.8 91.5 0.339 0.910 OK 0.795 0.421 OK 0.104 0.00	2 Warn	0.012	0.302	OK	0.340	0.390	ОК	0.280	0.238	92.1	91.8	92.7			16
19 92.4 92.0 0.976 0.554 OK 0.558 0.327 OK 0.279 0.07 20 92.7 93.1 93.6 0.560 0.404 OK 0.896 0.523 OK 0.151 0.02 21 92.6 91.8 91.5 0.339 0.910 OK 0.795 0.421 OK 0.104 0.07	0000000000000	0.014	0,357	ОК	0.374	0.396	oĸ	0.295	0.688	92.4	92.3	92.3			17
20 92.7 93.1 93.6 0.560 0.404 OK 0.896 0.523 OK 0.151 0.00 21 92.6 91.8 91.5 0.339 0.910 OK 0.795 0.421 OK 0.104 0.00	4 Warn	0.014	1	OK	0.451	0.550	Alert	0.003	0.858	92.9	927	92.7			18
21 92.6 91.8 91.5 0.339 0.910 OK 0.795 0.421 OK 0.104 0.00	\$00000000000000	0.015	•		0.327	0.558		0.554	0.976	92.0	92.4	92.4			19
	200000000000000000000000000000000000000	0.025	1	Commence of the Commence of th	i						200000000000000000000000000000000000000	100000000000000000000000000000000000000			
22 1 93 1 92 22 1 92 2 1 1 24 1 1 24 2 2 2 2 2 2 2 2	100000000000000000000000000000000000000	0.014	1		į.			1 1	•		*********				
	\$66000000000000000000000000000000000000	0.008	0.111	СK	0.534	0.730	ок	0.749	0.324	92.7	92.4	93.0			22
	************	0.020					************		1	distribution		identification de de la constantina de			
24 93.3 93.0 93.2 0.744 0.648 OK 0.838 0.506 OK 0.064 0.01	8 Watts	0.018	0.064	OK	0.506	0.838	OK	0.648	0.744	93.2	93.0	93.3			24

Figure SS-1

8

QC Data Verification Program

98.68

QL

Col 1

Version 1.004

PROGRAM FOR COMPARING CONTRACTOR QUALITY CONTROL FOR PAY & CDOT VERIFICATION DATA SETS 125, Hampden - South

PROJECT: IM0252-302 Location: Subaccount 11755 Begin Date: 8/3/97

5

ASPHALT CONTENT

Enter Data In Yellow Blocks Only

3

Statistical Data Based on Entries Below

Note: F-test compares differences in SD's, t-test differences in means.

12

5 Test Running Evaluation Verification Samples

13

14

15

16

Target 4.80 Contracto CDOT Probabilities Probability, Cumulative Probabilities Contractor QC CONTR. COM Verif F & t-test F & t-test F & t-test Verif 0.05 0.05 0.05 Mean 4.86 4.78 4.77 4.73 Warning SD 0.12 0.12 0.12 0.14 Alert 0.01 0.01 0.005 "n" 49 61 12 12 98,41 97.78

9

For Action on Verification For Info. on Verification Accumultive (For Final) Running Sets of 5 Samples Verification Cumulative Verification Contractor QC vs Verif Contractor's Independent Random Strata No. Quality Control Test Results Contracto CDOT F-test t-test Status F-test T-Test Status F-test T-Test Status 4,61 1 4.75 2 4.79 4.88 4.74 4.84 0.064 0.851 ОΚ 0.064 0.851 3 0.383 4.41 4 82 0.552 0.383 OK 0.552 OK 4 4.77 4.71 0.498 0.443 OK 0.498 0.443 ОK 5 0.126 OК 0.388 0.259 ОΚ 0.327 0.876 QΚ 6 4 65 4 84 4.59 4.75 4 63 4 75 4 58 4 58 4 79 0.100 4.82 4.86 4.70 4.77 4.65 4.80 4.81 4.79 0.038 0.206 Warn 0.272 0.272 OK 0.211 0.701 OK 4.60 0.153 0.400 0.617 ΟK 4.60 4.56 4.82 5.03 0.461 0.156 0.705 ΟK 8 4 84 4.88 QΚ 4.91 4.80 4.84 9 4.84 4.57 4.99 4.78 4.83 4,99 4.78 4.88 5.06 0.546 0.155 ЮK 0.958 0.087 bк 0.540 0.559 OK 0.764 0.041 0.750 0.280 OK 4.84 4.86 4.92 5.05 4.79 5.07 0.660 0.028 Warn Wam 4.83 4.85 10 4.95 11 4.86 4.90 4.89 4.86 4.89 4.78 4.69 4.71 4.81 0.140 0.043 Warn 0.742 0.027 Wam 0.743 0 145 DΚ 0.725 0.015 0.863 0.232 OK 12 4 74 4.72 4.60 4.71 4.77 4.80 4.57 4.75 4.89 0.291 0.004 Alert Warn

96 62

10

11

QC Data Verification Program

4.00

4.01

1.15

21

70.23

Target

Mean

SD

"n"

QL

Contractor QC

PROGRAM FOR COMPARING CONTRACTOR QUALITY CONTROL FOR PAY & CDOT VERIFICATION DATA SETS PROJECT: #M0252-302 Location: 1.25, Hampden & South

Subaccount: 11755 Begin Date:

Contractor

3.56

1.30

13

61.72

Verif

8/3/97

PERCENT AIR VOIDS

Enter Data In Yellow Blocks Only Statistical Data Based on Entries Below

CONTR. COMB

3.84

1.21

34

67.38

Note: F-test compares differences in SD's, t-test differences in means.

	5 Test Ru	nning Evaluation	Verification Samples	Contractor QC vs Verif
CDOT	1	Probabilities	Probability, Cumulative	Probabilities
Verif		F & t-test	F & t-test	F & t-test
4.02	Warning	0.05	0.85	0.05
0.65	Alert	0.01	D 91	0.005
12 95.18				

Col 1	2	3	4	5	6	7	8	9	10	11	12	13	14	15
						For Action	n on Verification		For Info.	on Verific	cation	Accumu	tive (For I	Final)
Strata	Contractor's I	ndependen	t Random	Verific	ation	Running	Sets of 5 Samples		Cumulat	ive Verifica	ation	Contract	or QC vs '	Verif
No.	ac	Test Resul	ts	Contr	CDOT	F-test	t-test	Status	F-test	T-Test	Status	F-test	T-Test	Status
1				3.40	4.78	1		F&t		1	F&t		İ	F&t
2				3.40	4,33				1	1			ļ	
3				4.40	4.48	0.272	0.172	ОК	0.272	0.172	OK		l	
4				4.00	4,34	0.197	0.102	ОK	0.197	0.102	OK	l	İ	
5				4.00	3.87	0.605	0.136	ОК	0.605	0.136	OK		l	
6	4.70	4.90	5.80	4 60	4.18	0.214	0.524	ОК	0.302	0.246	OK	0.661	0.016	Warn
7	4.30	5.10	4.70	4.80	4.37	0.445	0.491	СK	0.118	0.373	OK	0.873	0.017	Warn
8	3.60	2.80	5.20	4.30	4.55	0.530	0.656	ОЖ	0.104	0.303	ОK	0.161	0.229	CK.
9	4.50	5,90	4.40	4.30	3.58	0.664	0.152	ОК	0.416	0.546	OK	0.116	0.119	ОК
10	4.60	3,60	3:20	3.90	3.90	0.827	0.200	OK	0.460	0.543	ОK	0.058	0.231	ОК
11	4.30	3.10	2:20	3.20	3.39	0.735	0.490	ок	0.547	0.482	OK	0.037	0.464	Warn
12	3.20	2.00	2.20	2.00	2.41	0.685	0.902	OK	0.590	0.373	OK	0.166	0.679	οк

Figure SS-3

QC Da	ta Verifica	tion Progr	am				Version 1.004							
PROGR	AM FOR CO	MPARING C	ONTRACT	OR QUALITY	CONTRO	L FOR PA	Y & CDOT VERIFIC	CATION DA	TA SETS					
PROJE	ECT:	IM0252-30)2	Locat	ion:	125	Hampden - Sou	th	Š					
Subac	count:	11755		Begin Date	9 :	8/3/97		* *						
	PERCE	NT VOID	S in MI	NERAL A	GGRE	GATE (VMA)							
	Enter Data I	n Yellow Bio	cks Only			Note: F-tes	t compares differenc	es in SD's, t	test differe	nces in me	ans.			
		ata Based or	n Entries Be	low		5 Test Ru	nning Evaluation		Verificati	ion Sampl	es	Contract	tor QC vs	Verif
Target	14.50	-		Contractor	CDOT		Probabilities			ty, Cumula	ative	Probabili		
	ctor QC	CONTR.		Verif	Verif		F & t-test	s.	F & t-test	4		F & t-test	ā.	
Mean	14.64		14.54	14.37	14.45	Warning	0:05	55	0,05			0.05	ž.	
SD	0.57		0.53	0.42	0.38	Alert	0.01	B.	0.01	}		0.005	ș.	
"n"	21 96.88		33 98 12	12 99.90	12 100.00									
QL Col 1	2	3	4	55.50	6	7	8	9	10	11	12	13	14	15
2011					ľ		on Verification			on Verific			tive (For F	
Strata	Contractor's	Independent	t Random	Verific	ation	4	Sets of 5 Samples			ve Verifica			or QC vs \	'
No.	QC	Test Result	ts	Contr	CDOT	F-test	t-test	Status	F-test	T-Test	Status	F-test	T-Test	Status
1				14.10	20.00.000000000000000000000000000000000	1		F&t		l	F&t			F&1
2				14.00	14.50	1	1			İ				
3				14.30	14.40	1,,000	0.120	ОК	1.000	0.120	OK			
4				13.90	14.40	1	0.031	Warn	0.764	0.031	Warn		1	
5	14 40		40.00	14.30	14.00	0.509	0.172	OK	0.509	0.172	OK			
6	14.40	15.10 14.70		14.50 14.90	14:10 14:70	0.819	0.697	OK	0.751	0.393	OK	0.196	0.012	Warn
7	14.70	14.20	(***************	15.10	15.00	0.614 0.796	0.732 0.561	OK OK	0.600 0.510	0.501 0.554	OK OK	0.922 0.870	0.024	Warn
8 9	15.00	15.40		14.70	14.80	0.796	0.561	OK	0.510	0.554	OK	0.802	0.107	Warn
10	15.10	,	and purious Type	14.70	14.80	0.452	0.105	ok	0.523	0.494	OK	0.853	0.030	Warn
11	15.10	14.20		14.20	14.30	0.432	1.000	ок	0.523	0.387	ОК	0.456	0.060	OK
12	14.30	13,60	13.70	13.70	13.70	0.960	0.374	ОК	0.731	0.384	ОК	0.323	0.159	ОК

Figure SS-4

PROGRAM FOR COMPARING CONTRACTOR QUALITY CONTROL FOR PAY & CDOT VERIFICATION DATA SETS Location: PROJECT: NH 0021-022 Colo Blvd, Miss-MLK Blvd Subaccount 11600 Pr 3 Begin Date: 8/28/97 In-Place Density Enter Data In Yellow Blocks Only Target 94.00 nt compares diffe Probabilities in STVs. tatest differ CDOT Probability, Cumulative **Probabilities** Contracto CONTR. COMB Contractor QC Verif Verif F & t-test F & t-test 94.40 94.40 94.36 94.27 0,05 กกร 0.05 SD 0.61 0.69 0.69 0.78 Alert 0.03 0.005 0.005 "n" 22 22 99.90 49 54 98.69 QL 10 11 13 14 15 Col 1 1.0283 For Action on Verification For Info. on Verification Accumultive (For Final) Strata Contractor's Independent Rando Solit Verit Running Sets of 5 Samples Cumulative Verification Contractor QC vs Verif QC Resu QC Result QC Result T-Test DOT -test No. ontract -test t-test Status F-test T-Test 95,8 3 93.7 93.7 0.304 0.206 0.304 0.206 OK 95.4 95.6 0.786 0.248 OK 0.786 0.248 92.7 0.703 0.124 OK 0.703 0.124 95.6 OΚ 0.675 0.277 0.749 0.133 ΟЖ 94.4 94.6 OK 95.8 93.8 95.0 0.630 0.705 OK 0.951 0.289 OK 95.6 94.6 93.4 0.369 0.503 ĎК 0.836 0.181 ЮK 0.997 0.537 OK OR 0.463 0.730 0.163 ОК 0.693 0.476 9 95.4 95.4 95.2 0.434 OK 94.8 94.6 95.8 0.400 0.654 ОX 0.488 0.303 ОK 0.588 0.497 10 11 84.5 95.1 0.474 0.449 ΟK 0.526 0.439 OΚ 0.556 0.530 12 94.3 94.2 94.6 0.299 0.611 ОK 0.567 0.494 OK 0.603 0.608 QK 13 94.1 94.4 93.9 0.552 0.313 Ж 0.530 0.427 OK 0.731 0.756 OK оĸ 94.6 0.544 OK 94.3 95.1 0.028 0.126 0.505 0.638 0.725 14 Warn 0.486 0.501 0.825 0.925 15 B3.9 94.5 94.2 0.036 0.397 Warn 16 94.0 94.0 94.7 0.117 0.451 OΚ 0.546 0.607 aĸ 0.838 0.988 OK 17 94.5 94 5 97.8 0.646 1 000 OK. 0.479 0.509 OK 0.788 0.998 OK ОЖ ΟЖ 0.571 18 94.3 93.5 93.8 0.570 0.609 0.621 0.551 0.883 OK. 19 94.1 93.7 0.942 0.642 ОК 0.552 0.460 OK 0.592 0.971 94.4 93.9 94.3 0.761 0.784 OK 0.756 0.591 QΚ 0.487 0.926 QK 20 21 94.3 94.1 94.3 0.319 1.000 ΟК 0.777 0.619 OΚ 0.443 0.905 OK 22 94.4 94.4 93.5 0.596 0.910 OX 0.667 0.501 OK 0.412 0.911 OK. 23 0.774 0.541 0.336 OК 94.0 0.596 OK 0.855 93.5 93.8 0.910 OK ΟК 0.739 0.158 0.643 94.4 92.9 94.2 0.346 0.390 0.883 OK 25 94.0 94.4 92.5 0.842 0.734 ОК 0.697 0.508 OΚ 0.165 0.718 CK 26 93.0 93,4 94.3 0.866 0.924 OX 0.817 0.625 OX 0.362 0.842 OK 0.756 27 93.1 0.542 0.974 0.705 0.540 0.315

Version 1.004

OC Data Verification Program

Figure SS-5

QC Data Verification Program Version 1.004 PROGRAM FOR COMPARING CONTRACTOR QUALITY CONTROL FOR PAY & CDOT VERIFICATION DATA SETS NH 0021-022 Location: 11600, Pr 3 Begin Date: PROJECT: Colo Blvd, Miss-MLK Blvd Subaccount 8/28/97 ASPHALT CONTENT Enter Data in Yellow Blocks Only res differet n SD's, t-test differen 5 Test Running Evaluation Verification Samples Contractor QC vs Verif Statistical Data Based on Entries Below 4.80 CDO1 Probabilities Probability, Cumulative Probabilities Contracto Target CONTR. COMB F & t-test Contractor QC 4.77 4.69 0.05 0.05 0.05 SD 0.16 0.18 0.24 0.23 Alert 0.01 0.01 0.005 "n' 38 49 11 11 90.79 78.50 75.57 QL Çol 10 11 12 14 15 16 17 18 19 For Action on Verification For Info. on Verification Accumultive (For Final) Strata Contractor's Independent Random Verification Running Sets of 5 Samples Cumulative Verification Contractor QC vs Veril Quality Control Test Results Contracto CDOT r-Test No. -test t-test Status -test 5.23 4.82 0.184 3 4.65 4 65 0.341 0.184 OK 0.341 OK 4.52 0.811 0.058 0.811 0.058 OK 4.24 OK 0.707 0.037 0.707 0.037 Warr 4.77 4.64 4.53 4.59 4.61 5.02 4.93 4.65 4.65 0.790 0.113 ĐΚ 0.635 0.045 Warn 0 209 0.654 ÒК 0.739 0.613 0.017 Warn 0.019 4 68 4.85 4 84 4.B0 4.83 4.77 4.78 4.76 4.55 0.374 0.108 OK Warn 0.978 5.03 0.081 0.678 ÓK 0.907 0.219 0.004 4.88 4.80 4.92 4.70 4.92 4.75 4.75 4.68 0.727 OK OK 0.869 0.129 0.059 0.904 OK. 4.60 4.64 5,08 4.57 5.03 5.08 4,91 5.06 QK OK 10 4.65 4.79 5.03 5.09 4.90 4.86 4.75 4.79 4.61 0.267 0.627 0.868 0.080 0.066 0.735 0.259 OK 0.071 OK OK 0.902 0.338 11 4,90 4,60 4.69 4.5E 4.91 0.356 0.956

Figure SS-6

QC Data Verification Program Version 1.004 PROGRAM FOR COMPARING CONTRACTOR QUALITY CONTROL FOR PAY & CDOT VERIFICATION DATA SETS PROJECT: NH-0021-02 Location: Colo Bivd, Miss-MLK Bivd Subaccount: 11600; Pr.3. Begin Date: 8/28/97 PERCENT AIR VOIDS Note: F-test compares differences in SD's, t-test differences 5 Test Running Evaluation Verification Samples Enter Data in Yellow Blocks Only Statistical Data Based on Entries Below Contractor QC vs Verif Probability, Cumulative Target 4.00 Probabilities Contractor CDOT Probabilities Contractor QC CONTR. COM
Mean 4.44 4.52 Vent Verif F & t-test F & t-test F & t-test 0:05 0:01 0.05 4.01 4.63 Warning 0.05 SD "n" 0.73 0.70 0.005 0.68 0.62 Alert 27 82.81 16 11 84.12 79.77 96.55 QL 10 Col 1 5 6 8 11 12 13 14 15 For Action on Verification For Info. on Verification Accumultive (For Final) Contractor's Independent Ra Verif Running Sets of 5 Sample Cumulative Verification Contractor QC vs Verif ication No. QC Test Results FZI F&t F&t 2 5,20 0.859 0.003 0.859 0.003 5.90 4.84 0.942 0.000 0.942 0.000 5,30 5.13 0.674 5 0.008 Alert 0.674 0.008 4.35 4.60 0.865 Wash 6 7 0.026 0.697 0.008 Alert 0.709 OΚ 0.424 5.00 4.60 4,71 4.30 4,18 0.549 0.069 OK 0.836 0.008 Alert 0.151 0.543 OK. 5.60 4.90 4.70 4.00 3.67 0.578 0.084 0.999 0.005 0.175 0,816 9 5.50 3.70 3.60 3,40 4.31 0.583 0.987 ΟК 0.542 0.069 OK 0.528 0.811 4.50 10 4.10 4.30 2,90 3.40 0.799 0.601 ОK 0.675 0.032 Warn 0.983 0.736 OK 3.50 4.90 3,40 11 0.602 0.359 ОЖ 0.768 0.014 Warn

Figure SS-7

PRO.	ECT:	OMPARI NH 00	NG CONT											
								L CDOT VE		ON DATA	SETS			
Bau							T T	s-MLK BI	vd	8				
	**********			Begin D		8/28/9); 20 <u>00</u> 0202000000	>					
	PERC	ENTY	DIDS i	n MINE	RAL AC	GREG	ATE (V	MA)						
		a in Yellow		•		Note: F-te	st compares	differences	in SD's, t-	test differ	nces in me	MW.		
		Data Base	ed on Entr			5 Test Ru	inning Eval	uation	Verificat	ion Sampl	es	Contrac	tor QC vs	: Verif
	14.50			Contractor		1	Probabilit	ies	Probabilit	ty, Cumula	tive	Probabili	ties	
ontr	actor QC	CONTR	. COM	Verif	Verif		F & t-test		F & t-test	۸.		F & t-tes	ţ.	
ean	14.83		14.89	14.98	14,44	Warning	0.05	į.	0.05	ŧ		0,05	ĝ	
)	0.36		0.33	0.26	0.26	Alert	0.01	ŭ.	0.01	i.		0.009	Ř	
)"	16		27	11	11									
L	99.59	3	99.54	100.00	100.00									
ol 1	2	3	4	5	6	7	8	9	10	11	12	13	14	15
					<u> </u>	For Action	on Verifica	tion	For info.	on Verific	ation	Accumult	ive (For F	inal)
rata	1	r's indepen					Sets of 5 S			re Verificat		-	or QC vs '	_
No.	QC	Test Resu	ılts	Contr	CDOT	F-test	t-test	Status	F-test	T-Test	Status	F-test	T-Test	Statu
1				15.30	14.40		ł	F&1			F&l	l	1	F 8 t
2				15,20	14.40	ł	1		1			1		
3				15,40	14.10	1 0.000	0.023	Warn	0.500	0.023	Warn		1	
4				15.20	14.90	0.071	0.028	Warn	0.071	0.028	Warn	1	1	
5				14.90	14.50	0.423	0.015	Warn	0.423	0,015	Warn	1	l	
6	14.60	14.90	14.20	14,70	14,10	0.738	0.018	Warn	0.804	0.005	Alert	0.524	0.032	Wa
7	15,00	14.90	15.00	15.00	14.20	0.653	0.018	Warn	0.746	0.001	Alert	0.560	0.055	OH.
8	15,60	14.70	14.90	14.70	14.8D	0.346	0.058	OK .	0.761	0.004	Alert	0.391	0.269	- OK
9	15.50 14.60	14.30	14.90	14.70	14.60	0.197	0.092	OK	0.908	0.005	Alert	0.272	0.401	O#
-		14.90	14.60	14.90	14,30	0.190	0.079	OX	0.891	0.002	Alert	0.281	0.255	OR
10	14.70			14.80	14.50	0.268	0.105	OK	0.947	0.001	Alert			

Figure SS-8

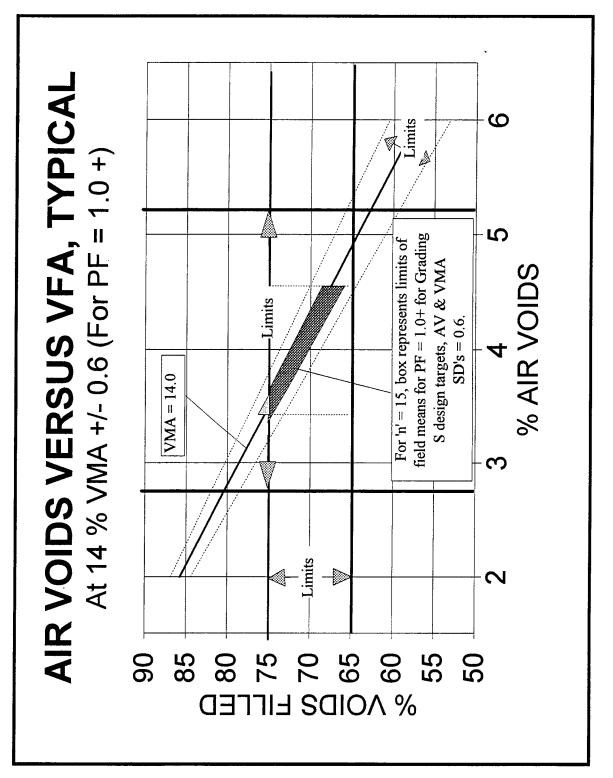


Figure 1

--- Cnt:Verif/QC Cuml

--- Cntr/CDOT, Cuml

- Cnt/CDOT, Rning 5

Figure 3

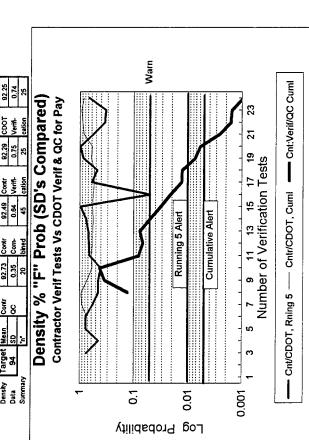


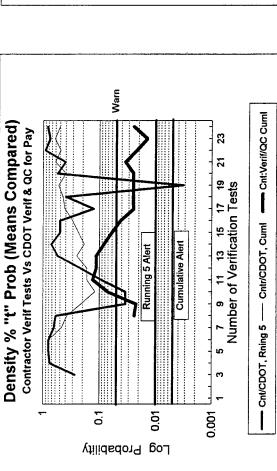
ater than 2 x V below lower Tolerance limit)

IN-PLACE ASPHALT DENSITY: 125, HAMPDEN SOUTH

(Excludes Verifi

Density % "F" Prob (SD's Compared) Contractor Verif Tests Vs CDOT Verif & QC for Pay

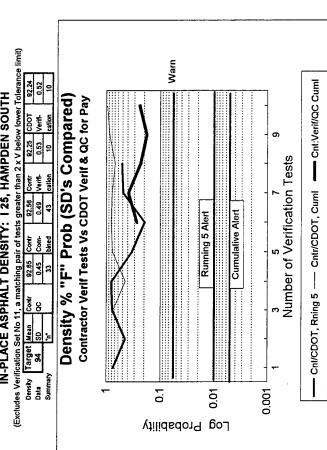




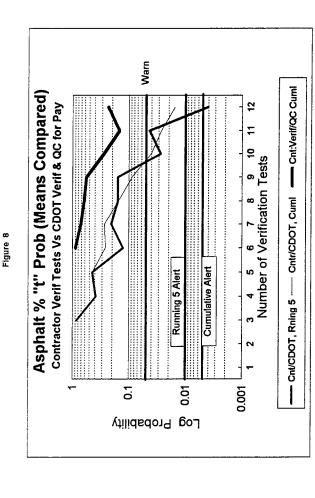
Warn Warn - Cnt:Verif/QC Cuml Density % "t" Prob (Means Compared) Contractor Verif Tests Vs CDOT Verif & QC for Pay 23 Number of Verification Tests 15 17 Cntr/CDOT, Cuml Cumulative Alert Running 5 Alert Cumulative Alert Figure 4 Cnt/CDOT, Rning 5 0.0 0.001 0.1 0.01 0.001 0.1 Log Probability Log Probability

REDUCED VERIFICATION, 1 Ver To 7 QC IN-PLACE ASPHALT DENSITY: I 25, HAMPDEN SOUTH

ASPHALT CONTENT: 125, HAMPDEN SOUTH



| Asphate | Target | Mean | Cont. | 4.73 | Cont. | 4.73 | Cont. | 4.74 | Cont. | 4.74 | Cont. | 4.75 | Cont. | 4.75 | Cont. | 4.75 | Cont. | 4.75 | Cont. | 4.75 | Cont. | 4.75 | Cont. | 4.75 | Cont. | 4.75 | Cont. | 4.75 | Cont. | 4.75 | Cont. | 4.75 | Cont. | 4.75 | Cont. | 4.75 | Cont. | 4.75 | Cont. | 4.75 | Cont. | 4.75 | Cont. | 4.75 | Cont. | 4.75 | Cont. | 4.75 | Cont. | 4.75 | Cont. | 4.75 | Cont. | 4.75 | Cont. | 4.75 | Cont. | 4.75 | Cont. | 4.75 | Cont. | 4.75 | Cont. | 4.75 | Cont. | 4.75 | Cont. | 4.75 | Cont. | 4.75 | Cont. | 4.75 | Cont. | 4.75 | Cont. | 4.75 | Cont. | 4.75 | Cont. | 4.75 | Cont. | 4.75 | Cont. | 4.75 | Cont. | 4.75 | Cont. | 4.75 | Cont. | 4.75 | Cont. | 4.75 | Cont. | 4.75 | Cont. | 4.75 | Cont. | 4.75 | Cont. | 4.75 | Cont. | 4.75 | Cont. | 4.75 | Cont. | 4.75 | Cont. | 4.75 | Cont. | 4.75 | Cont. | 4.75 | Cont. | 4.75 | Cont. | 4.75 | Cont. | 4.75 | Cont. | 4.75 | Cont. | 4.75 | Cont. | 4.75 | Cont. | 4.75 | Cont. | 4.75 | Cont. | 4.75 | Cont. | 4.75 | Cont. | 4.75 | Cont. | 4.75 | Cont. | 4.75 | Cont. | 4.75 | Cont. | 4.75 | Cont. | 4.75 | Cont. | 4.75 | Cont. | 4.75 | Cont. | 4.75 | Cont. | 4.75 | Cont. | 4.75 | Cont. | 4.75 | Cont. | 4.75 | Cont. | 4.75 | Cont. | 4.75 | Cont. | 4.75 | Cont. | 4.75 | Cont. | 4.75 | Cont. | 4.75 | Cont. | 4.75 | Cont. | 4.75 | Cont. | 4.75 | Cont. | 4.75 | Cont. | 4.75 | Cont. | 4.75 | Cont. | 4.75 | Cont. | 4.75 | Cont. | 4.75 | Cont. | 4.75 | Cont. | 4.75 | Cont. | 4.75 | Cont. | 4.75 | Cont. | 4.75 | Cont. | 4.75 | Cont. | 4.75 | Cont. | 4.75 | Cont. | 4.75 | Cont. | 4.75 | Cont. | 4.75 | Cont. | 4.75 | Cont. | 4.75 | Cont. | 4.75 | Cont. | 4.75 | Cont. | 4.75 | Cont. | 4.75 | Cont. | 4.75 | Cont. | 4.75 | Cont. | 4.75 | Cont. | 4.75 | Cont. | 4.75 | Cont. | 4.75 | Cont. | 4.75 | Cont. | 4.75 | Cont. | 4.75 | Cont. | 4.75 | Cont. | 4.75 | Cont. | 4.75 | Cont. | 4.75 | Cont. | 4.75 | Cont. | 4.75 | Cont. | 4.75 | Cont. | 4.75 | Cont. | 4.75 | Cont. | 4.75 | Cont. | 4.75 | Cont. | 4.75 | Cont. | 4.75 | Cont. | 4.75 | Cont. | 4.75 | Cont. |



VOIDS MINERAL AGGREGATE: 125, HAMPDEN SOUTH

/oids	Target	Mean	Contr	14.64	Contr	14.54	Confr	14.37 CDOT	CDOT	14.45
Aineral	14.5	SD	ဗ	0.57	Corp	0.53	Verif	0.42	Verif	0.38
Aggr. Sun	nmany	•		21	pined	33	cation	12	cation	12

0.39 9

14.28

14.54 0.53 33

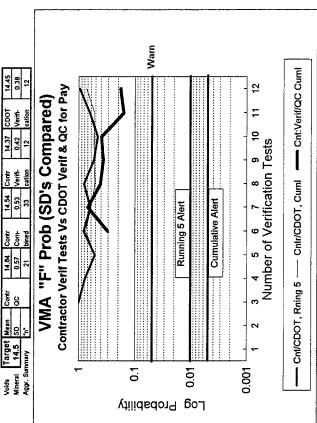
14.64 Contr 0.53 Com-

Mean Contr SD QC "n"

Target 14.5 Voids Target Mineral 14.5 Aggr. Summary Contractor Verif Tests Vs CDOT Verif & QC for Pay

VMA "F" Prob (SD's Compared)

REDUCED VERIFICATION, 1 Ver To 7 QC VOIDS MINERAL AGGREGATE: 125, HAMPDEN SOUTH



Warn

Figure 10

--- Cnt:Verif/QC Cum

--- Cntr/CDOT, Cuml

--- Cnt/CDOT, Rning 5

Figure 12

Number of Verification Tests

0.001

Cumulative Alert

Running 5 Alert

0.0

0.

Log Probability

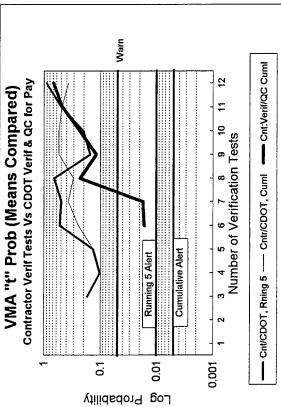
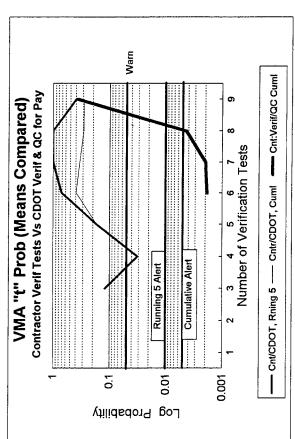
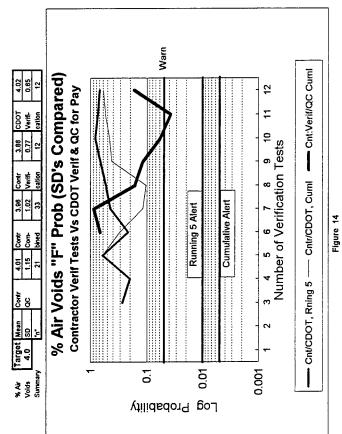
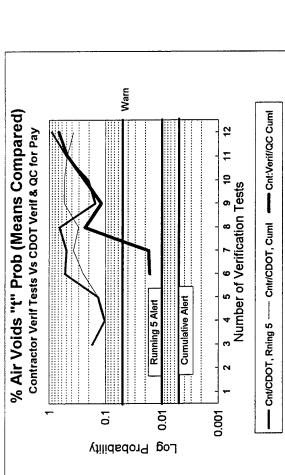


Figure 13



PERCENT AIR VOIDS: 125, HAMPDEN SOUTH





REDUCED VERIFICATION, 1 Ver To 7 QC PERCENT AIR VOIDS: 125, HAMPDEN SOUTH

	_	Warn		E
% Av Target Mean Confr 4.05 Confr 3.36 Confr 3.70 CDOT 3.93 Voids 4.0 5D QC 1.08 Conn 1.02 Veriff 0.80 Veriff 0.72 Surmmary 'n' 24 bised 33 cation 9 cation 9	% Air Voids "F" Prob (SD's Compared) Contractor Verlf Tests Vs CDOT Verlf & QC for Pay	0.1	 0.001	Cnt/CDOT, Rning 5 —— Cntr/CDOT, Cuml Cnt:Verif/QC Cuml
% Air Voids Summa		bability	O	

Figure 16

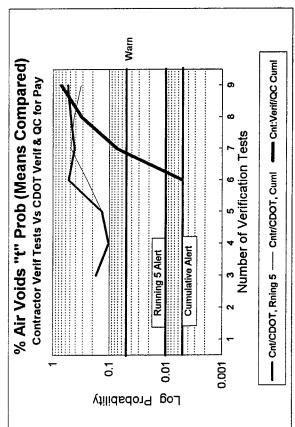


Figure 17

IN-PLACE ASPHALT DENSITY: COLO BLVD, MISS - MLK BLVD

Density	Target	Irget Mean	Contr	94.40 Contr	Confr	94.36 Contr	Confr	94.33	CDOT	24.21
Data	8	SD	ခ	0.61	Comp	69.0	Verif	0.75		0.81
Summany		·u.		22	bined	49	cation	27	cation	27
•	I						l			

Density % "F" Prob (SD's Compared)

Contractor Verif Tests Vs CDOT Verif & QC for Pay

Contractor Verif Tests Vs CDOT Verif & QC for Pay

Contractor Verif Tests Vs CDOT Verif & QC for Pay

Warm

Contractor Verif Tests Vs CDOT Compared)

Warm

Contractor Verif Tests Vs CDOT Compared

Warm

Contractor Verification Tests

Figure 18

Contractor Verif Tests Vs CDOT Verif & QC for Pay

Contractor Verif Tests Vs CDOT Verif & QC for Pay

Contractor Verif Tests Vs CDOT Verif & QC for Pay

Contractor Verif Tests Vs CDOT Verif & QC for Pay

Contractor Verif Tests Vs CDOT Verif & QC for Pay

Marm

Contractor Verif Tests Vs CDOT Verif & QC for Pay

Contractor Verif Tests Vs CDOT Verif & QC for Pay

Marm

Contractor Verif Tests Vs CDOT Verif & QC for Pay

Contractor Verif Tests Vs CDOT Verif & QC for Pay

Contractor Verif Tests Vs CDOT Verif & QC for Pay

Contractor Verif Tests Vs CDOT Verif & QC for Pay

Marm

Contractor Verif Tests Vs CDOT Verif & QC for Pay

Contractor Verif Tests Vs CDOT Verif & QC for Pay

Contractor Verif Tests Vs CDOT Verif & QC for Pay

Contractor Verif Tests Vs CDOT Verif & QC for Pay

Contractor Verif Tests Vs CDOT Verif & QC for Pay

Contractor Verif Tests Vs CDOT Verif & QC for Pay

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Contractor Verif Tests Vs CDOT Verif & QC for Pay

Contractor Verif Tests Vs CDOT Verif & QC for Pay

Contractor Verif Tests Vs CDOT Verif & QC for Pay

Contractor Verif Tests Vs CDOT Verif & QC for Pay

Contractor Verif Tests Vs CDOT Verif & QC for Pay

Contractor Verif Tests Vs CDOT Verif Tests Vs CDOT Verif Tests Vs CDOT Verif Tests Vs CDOT Vs

Figure 19

Cnt:Verif/QC Cuml

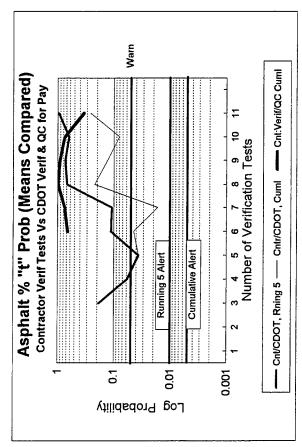
--- Cnl/CDOT, Rning 5 --- Cntr/CDOT, Cuml

0.001

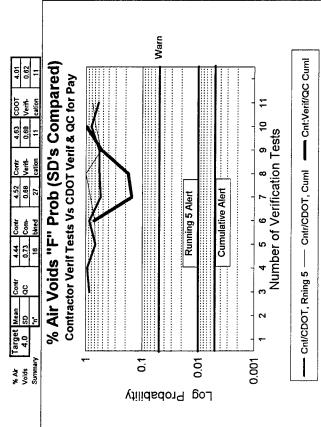
ASPHALT CONTENT: COLO BLVD, MISS - MLK BLVD

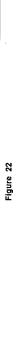
Log Probability

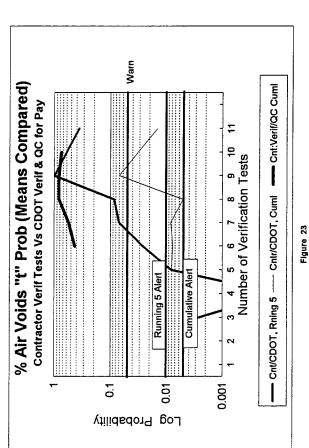
Figure 20



PERCENT AIR VOIDS: COLO BLVD, MISS - MLK BLVD







VOIDS MINERAL AGGREGATE: COLO BLVD, MISS - MLK BLVD

Copy 14.44 Copy 14.44 Copy Warn		-14	:Verif/QC Cuml	
Volds Target Mean Control of Letts 14.83 Control of	Vobability 2.9	90 0.01 Running 5 Alert Cumulative Alert	0.001 1 2 3 4 5 6 7 8 9 10 Number of Verification Tests	— Cnl/CDOT, Rning 5 — Cntr/CDOT, Cuml — Cnt:Verif/QC Cuml

lqure 24

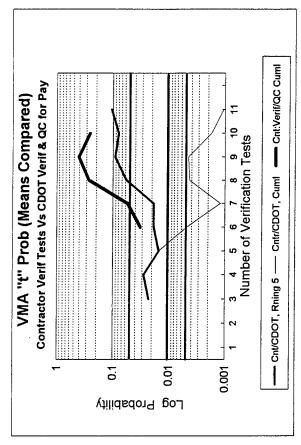


Figure 25

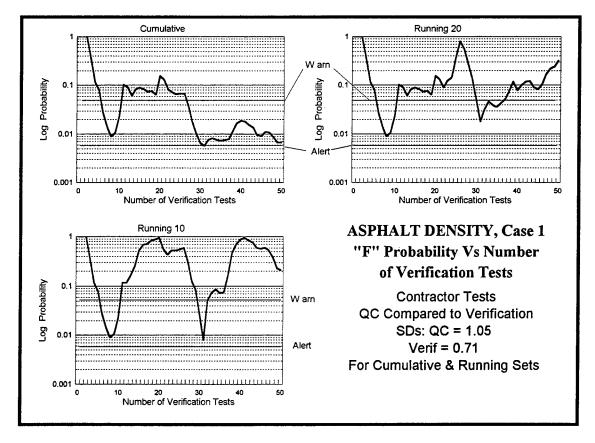


Figure 26

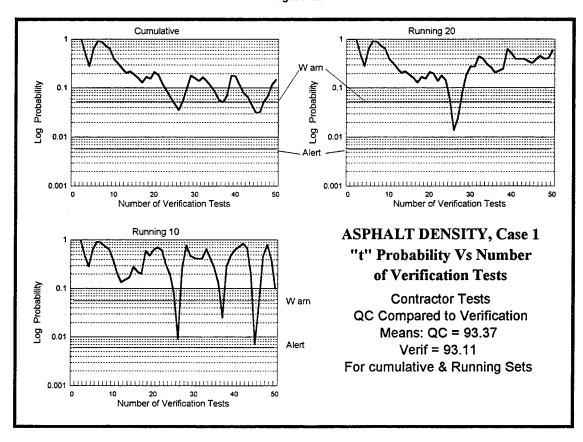


Figure 27

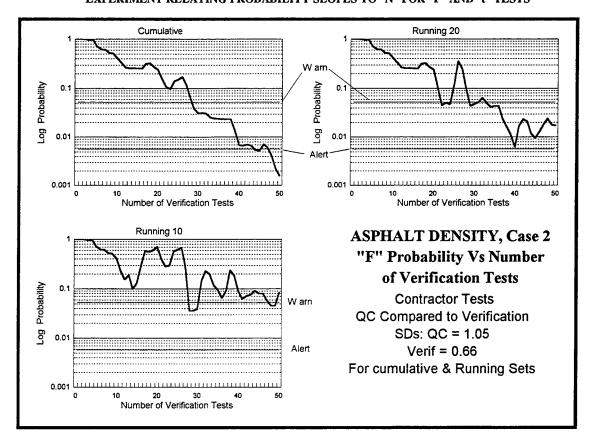


Figure 28

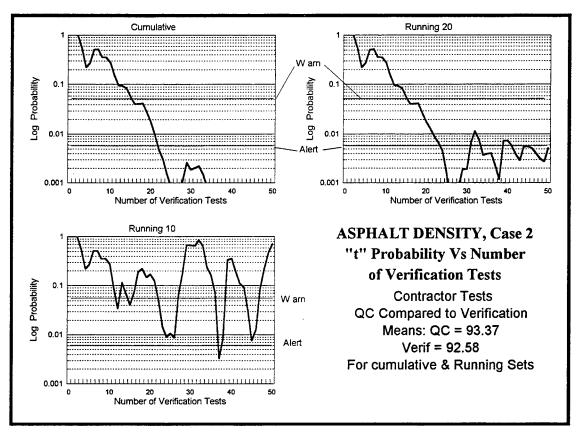


Figure 29

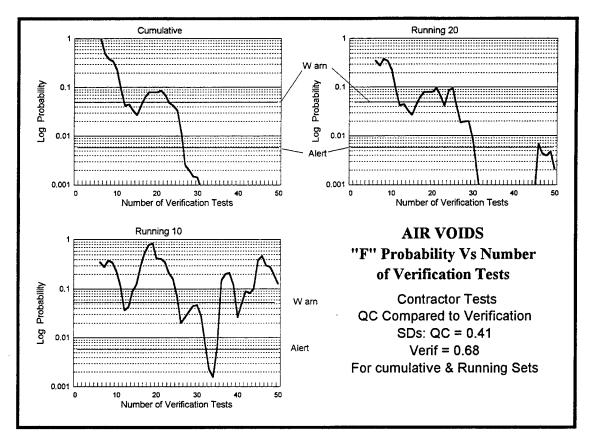


Figure 30

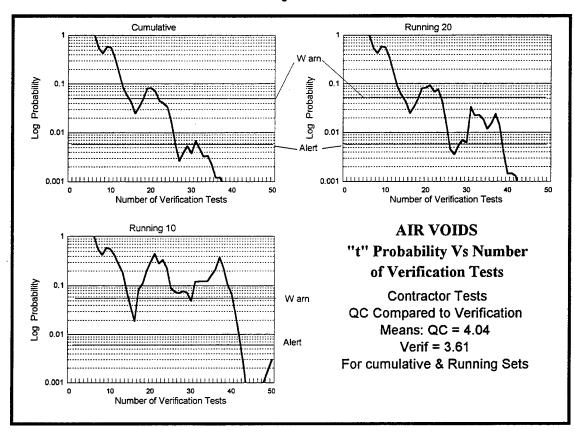


Figure 31

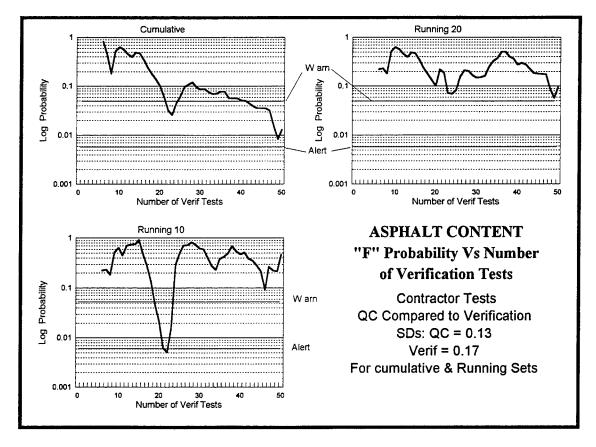


Figure 32

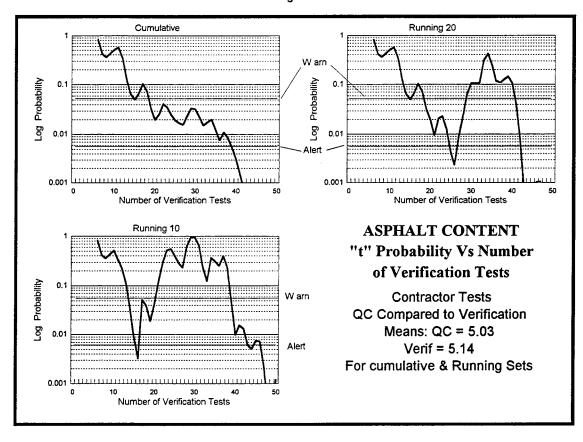


Figure 33

May 5, 1997

REVISION OF SECTIONS 105 AND 106 VOIDS ACCEPTANCE & QUALITY OF HOT BITUMINOUS PAVEMENT

Sections 105 and 106 of the Standard Specifications are hereby revised for this project as follows:

Subsection 105.03 shall include the following:

Conformity to the Contract of all Hot Bituminous Pavement, Item 403, except Hot Bituminous Pavement (Patching), Furnish Hot Bituminous Pavement and temporary pavement will be determined by tests and evaluations of asphalt content, gradation, in-place density, air voids and voids in the mineral aggregate in accordance with the following:

All work performed and all materials furnished shall conform to the lines, grades, cross sections, dimensions, and material requirements, including tolerances, shown in the Contract. For those items of work where working tolerances are not specified, the Contractor shall perform the work in a manner consistent with reasonable and customary manufacturing and construction practices.

When the Engineer finds the materials or work furnished, work performed, or the finished product are not in conformity with the Contract and has resulted in an inferior or unsatisfactory product, the work or material shall be removed and replaced or otherwise corrected at the expense of the Contractor.

Materials will be sampled randomly and tested by the Contractor and the Department in accordance with Sections 106 and 403 and with the applicable procedures contained in the Department's Field Materials Manual. The approximate maximum quantity represented by each sample will be as set forth in Section 106. Additional samples may be selected and tested as set forth in Section 106 at the Engineer's discretion.

A process will consist of a series of values resulting from tests of the Contractor's work and materials. Each process will consist of one or more test results. All materials produced will be assigned to a process. A process normally will include all materials produced prior to a change in the job mix formula (CDOT form 43). The Engineer will establish a new process when job mix formula changes occur. The Engineer may separate a process in order to accommodate small quantities or unusual variations.

Evaluation of materials for pay factors (PF) will be done using either the Contractor's quality control test results or the Department's verification test results. Each process will have a PF computed in accordance with the requirements of this Section. Test results determined to have sampling or testing errors will not be used.

Any of the Contractor's Quality Control test results for asphalt content, gradation or in-place density greater than the distance 2 x V (see Table 105-1) outside the tolerance limits will be designated as a separate process and the quantity it represents will be evaluated in accordance with subsection 105.03(g). If the material is permitted to remain in place, the PF for the item will not be greater than 0.75.

In the case of in-place density, the Contractor will be allowed to core the exact location of a Quality Control test result more than 2 x V outside the tolerance limit. The result of this core shall be used in lieu of the previous test result. All costs associated with coring will be at the Contractor's expense.

(a) Representing Small Quantities. When it is necessary to represent a process for asphalt cement, gradation or in-place density by only one or two tests results, PF will be the average of PFs resulting from the following:

If the test result is within the tolerance limits then PF = 1.00

If the test result is above the maximum specified limit, then

REVISION OF SECTIONS 105 AND 106 VOIDS ACCEPTANCE & QUALITY OF HOT BITUMINOUS PAVEMENT

If the test result is below the minimum specified limit, then

$$PF = 1.00 - 0.25[(T_L - T_o)/V]^2$$

Where: PF = pay factor.

V = V factor from Table 105-1.

 T_o = the individual test result.

 T_u = upper specification limit.

 T_L = lower specification limit.

If the pay factor of any of the above calculations is less than 0.75 for asphalt content, gradation, or in-place density, the acceptance of the work will be evaluated according to subsection 105.03(g).

- (b) Determining Quality Level. Each process with three or more test results will be evaluated for a quality level (QL) in accordance with Colorado Procedure 71.
- (c) Gradation Element. Each specified sieve will be evaluated for QL separately. The lowest QL for any specified sieve will be designated as the QL for gradation element for the process.
- (d) Element Pay Factor. Using QL, compute PF, as follows: For asphalt content, gradation and in-place density, the number of random samples (Pn) in each process will determine the pay factor for each element. As test results are accumulated, Pn will change accordingly. When the process has been completed, the number of samples it contains will determine the calculation of PF, based on the formula designated in Table 105-2. Where Pn is greater than 9 and less than 201, PF will be computed by the following formula:

$$PF = \frac{(PF_1 + PF_2)}{2} + \frac{[(PF_2 + PF_3) - (PF_1 + PF_2)]}{2} \times \frac{(Pn_2 - Pn_x)}{(Pn_2 - Pn_3)}$$

Where, when referring to Table 105-2:

PF₁ = PF determined at the next lowest Pn formula using process QL.

 $PF_2 = PF$ determined using the PN formula shown for the process QL.

PF₃ = PF determined at the next highest Pn formula using process QL.

 Pn_2 = the lowest Pn in the spread of values listed for the process Pn formula.

 Pn_3 = the lowest Pn in the spread of values listed for the next highest Pn formula.

 Pn_x = the actual number of test values in the process.

Regardless of QL, the maximum PF in relation to Pn is limited according to Table 105-2. For air voids and voids in the mineral aggregate, use the following formula for each process:

$$PF = 0.01619 - 0.14857(QL/100) + 0.15238(QL/100)^{2}$$

Where: PF = pay factor.

QL = Quality Level

(e) Element Average Pay Factor. A pay factor will be determined for all material or work represented by the elements listed in Table 105-1. For the pay estimates, each individual element will have the average pay factor (PF_A), weighted by the quantities, computed as follows:

$$PF_A = \frac{[M_1(PF_1) + M_2(PF_2) +M_j(PF_j)]}{\sum M}$$

Where: M_i = Quantity of item represented by the process.

 $P\tilde{F}_{j}$ = The process pay factor. $\sum M = \text{Sum of Quantities}, M_{1} \text{ to } M_{j} \text{ (the total quantity)}.$

(f) Composite Pay Factor. When there is more than one element for the item, determine the composite pay factor (PF_C) as follows (at project completion, $\sum M$ used to compute each element PF_A must be numerically the same):

$$PF_{c} = \frac{[W_{1}(PF_{A1}) + W_{2}(PF_{A2}) +W_{j}(PF_{Aj})]}{\sum W}$$

Where: W = element factor from Table 105-1.

 PF_{Aj} = element average pay factor.

 $\sum W = \text{sum of the element factors.}$

The composite pay factor for air voids and voids in the mineral aggregate will be computed separately and then added to the composite pay factor for asphalt content, gradation and in-place density. When the composite pay factor for air voids and voids in the mineral aggregate computes to a value less than zero, then the composite pay factor for air voids and voids in the mineral aggregate will be zero.

As the Contractor's verification and quality control test results become available, they will be used to calculate accumulated QL and PF numbers for each element and for the item. The test results and the accumulated calculations will be made available to the Engineer upon request. Numbers from the calculations will be carried to significant figures and rounded according to AASHTO Standard Recommended Practice R-11.

- (g) Evaluation of Work. When the PF_A for all elements in a process are 0.75 or greater, the finished quantity of work represented by the process will be accepted at the appropriate pay factor. If PF_A for asphalt content, gradation or in-place density is less than 0.75, the Engineer may:
 - Require complete removal and replacement with specification material at no additional cost to the Department; or
 - where the finished product is found to be capable of performing the intended purpose and the value of the finished product is not affected, permit the Contractor to leave the material in place.

If the material is permitted to remain in place the PF_C for the item will not be greater than 0.75. When condition red, as described in Section 106, exists for any element, resolution and correction will be in accordance with Section 106. Material which the Engineer determines is obviously defective may be isolated and rejected without regard to sampling sequence or location within a process.

Table 105-1 "W" and "V" Factors For Various Elements

Element	V factor	W factor
No. 8 mesh and larger sieves	2.80	N/A
No. 30 mesh sieve	1.80	N/A
No. 200 mesh sieve	0.80	N/A
Sieve analysis	N/A	20
Asphalt content	0.20	30
In-place Density	1.10	50
Voids in the Mineral	0.60	50
Air Voids	0.60	50

Table 105-2 Formulas For Calculation PF Based on P_n

Pn	When Pn is as shown at left is 3 to 9, or greater than	Maximum PF
3	0.31177 + 1.57878 (QL/100) - 0.84862 (QL/100) ²	1.025
4	0.27890 + 1.51471 (QL/100) - 0.73553 (QL/100) ²	1.030
5	$0.25529 + 1.48268 \text{ (QL/100)} - 0.67759 \text{ (QL/100)}^2$	1.030
6	$0.19468 + 1.56729 (QL/100) - 0.70239 (QL/100)^2$	1.035
7	$0.16709 + 1.58245 \text{ (QL/100)} - 0.68705 \text{ (QL/100)}^2$	1.035
8	$0.16394 + 1.55070 \text{ (QL/100)} - 0.65270 \text{ (QL/100)}^2$	1.040
9	$0.11412 + 1.63532 \text{ (QL/100)} - 0.68786 \text{ (QL/100)}^2$	1.040
10 to 11	0.15344 + 1.50104 (QL/100) - 0.58896 (QL/100) ²	1.045
12 to 14	$0.07278 + 1.64285 \text{ (QL/100)} - 0.65033 \text{ (QL/100)}^2$	1.045
15 to 18	0.07826 + 1.55649 (QL/100) - 0.56616 (QL/100) ²	1.050
19 to 25	$0.09907 + 1.43088 (QL/100) - 0.45550 (QL/100)^2$	1.050
26 to 37	0.07373 + 1.41851 (QL/100) - 0.41777 (QL/100) ²	1.055
38 to 69	0.10586 + 1.26473 (QL/100) - 0.29660 (QL/100) ²	1.055
70 to 200	0.21611 + 0.86111 (QL/100)	1.060
≥ 201	0.15221 + 0.92171 (QL/100)	1.060

Subsection 106.03 shall include the following:

All Hot Bituminous Pavement, Item 403, except Hot Bituminous Pavement (Patching), Furnish Hot Bituminous Pavement and temporary pavement shall be tested in accordance with the following program of process control testing and acceptance testing:

- (a) Quality Control Testing. The Contractor shall be responsible for Quality Control testing on elements as listed in Table 106-1. Quality Control sampling and testing shall be performed at the expense of the Contractor. The Contractor shall develop a quality control plan (QCP) in accordance with the following:
 - 1. Quality Control Plan. For each element listed in Table 106-1, the QCP must provide adequate details for assurance of process control. The Contractor shall submit the QCP to the Engineer at the preconstruction conference. The Contractor shall not start any work on the project until the Engineer has approved the OCP in writing.
 - A. Frequency of Tests or Measurements. The QCP shall include a schedule showing the locations of samples based on a random stratified sampling frequency, which shall not be less than that shown in Table 106-1.
 - B. Test Result Chart. Each quality control test result, the appropriate tonnage and the tolerance limits shall be plotted. For in-place density tests, only results after final compaction shall be shown. The chart shall be posted daily at a location convenient for viewing by the Engineer.
 - C. Quality Level Chart. The Quality Level (QL) for each quality control element in Table 106-1 and each required sieve size shall be plotted. The QL will be calculated in accordance with the procedure in CP 71 for Determining Quality Level (QL). The QL will be calculated on tests 1 through 3, then tests 1 through 4, then tests 1 through 5, then thereafter the last five consecutive test results. The tonnage of material represented by the last test result shall correspond to the QL. For in-place density tests, only results after final compaction shall be shown. The chart shall be posted daily at a location convenient for viewing by the Engineer.
 - D. F-test and t-test Charts. The results of F-test and paired sample t-test analysis between the Department's verification tests and the Contractors verification tests shall be shown on charts. Another chart shall show the results of F-test and t-test analysis assuming equal variances between the Contractor's verification tests and the Contractor's quality control tests. Each element in Table 106-1 and each required sieve size shall be plotted. The F-test and t-test will be calculated in accordance standard statistical procedures. The F-test and t-test will be calculated on tests 1 through 5, then thereafter the last five consecutive test results. The tonnage of material represented by the last test result shall correspond to the F-test and t-test. A warning value of 5% and an alert value of 1% shall be shown on each chart. For in-place density tests, only results after final compaction shall be shown. The chart shall be posted daily at a location convenient for viewing by the Engineer.
 - 2. Point of Sampling. The material for verification and quality control testing shall be sampled by the Contractor using approved procedures as designated in Section 403. Acceptable procedures are Colorado Procedures. The location where material samples will be taken shall be indicated in the QCP. Both the Contractor's verification tests and the Department's verification tests shall be sampled together at the same location and time. The Engineer shall perform the splitting of samples for verification tests.

- 3. Testing Standards. The QCP shall indicate which testing standards will be followed. Acceptable standards are Colorado Procedures.
- 4. Testing Supervisor Qualifications. The person responsible for the Quality Control testing shall be identified in the QCP. This person must possess one or more of the following qualifications:
 - A. Registration as a Professional Engineer in the State of Colorado.
 - B. Level A, B, and C certifications from the Laboratory Certification for Asphalt Technicians (LabCAT).
- 6. Technician Qualifications. Technicians taking samples and performing tests must possess the following qualifications:
 - A. Technicians taking samples and conducting compaction tests must have Level II A certification from the LabCAT.
 - B. Technicians conducting tests of asphalt content and gradation tests must have Level II B certification from the LabCAT.
 - C. Technicians determining asphalt mixture volumetrics and strength characteristics must have Level II C certification from the LABCAT.
- Testing Equipment. Equipment to be used for conducting the Contractor's verification and quality control tests shall be verified in the laboratory intended for use on the project. Equipment verification is intended to identify whether actual apparatus used meets the requirements of this section before testing begins. The Contractor's equipment verification will be conducted by the Colorado Asphalt Paving Association (CAPA). The Contractor shall arrange for verification of the laboratory with enough advance notice so that construction is not delayed. The person responsible for quality control testing and the technicians who will be taking samples and conducting quality control tests are required to attend the verification. The Department's Independent Assurance Tester should also attend. The laboratory shall be assembled and operating as though actual testing were underway when the verification process occurs. Items to be verified are listed on the LabCAT Laboratory Inspection Form. The verification shall be documented on the LabCAT Laboratory Inspection Form and a copy will be provided for the Contractor and the Engineer. All costs for conducting a verification of equipment and laboratory shall be at the Contractor's expense and shall not exceed \$450 per trip. Equipment and Laboratory verification will be valid for more than one project if the laboratory does not relocate and the equipment has not been idle for more than 30 days. All of the testing equipment used to conduct quality control testing shall conform to the standards specified in the test procedures and be in good working order. Calibration of the Contractor's nuclear testing devices used for testing of in-place density is a responsibility of the Contractor and shall not be conducted on the Department's calibration blocks.
- 8. Reporting and Record Keeping. The Contractor shall report the results of the tests to the Engineer in writing at least once per day. The Contractor shall make provisions such that the Engineer can inspect quality control work in progress, including sampling, testing, plants, documentation and the Contractor's testing facilities at any time. The engineer will provide results of the Department's verification tests within on working day.

(b) Verification Testing. Verification testing is the responsibility of the Contractor and the Department according to Table 106-1. The Department will determine the locations where samples or measurements are to be taken and as designated in Section 403. The maximum quantity of material represented by each test result and the minimum number of test results shall be in accordance with Table 106-1. The location or time of sampling shall be based on a stratified random procedure. Verification sampling and testing procedures will be in accordance with the Schedule for Minimum Materials Sampling, Testing and Inspection in the Department's Field Materials Manual. Samples for verification and acceptance testing shall be taken by the Contractor in accordance with the designated method. The samples shall be taken in the presence of the Engineer. Splitting of verification samples will be performed by the Engineer.

All materials being used are subject to inspection and testing at any time prior to, during, or after incorporation into work. All test results shall be reported directly to the Engineer without prior exchange of information between persons performing the tests. During production, results from split samples of the verification tests will be compared using the paired sample t-test and F-test statistical methods on the five most recent test results. As another test result becomes available, another analysis shall be performed. If an analysis results in a value between 5% and 1%, then a warning exists and the persons performing the tests shall meet to discuss reasons for the warning and solutions to the discrepancy. If an analysis results in a value of 1% or less, then an alert exists and condition red exists. The Engineer will meet with the Contractor to discuss reasons for the alert and recommend actions to be taken.

An analysis of test results will be performed after all test results are known using the t-test and F-test statistical methods. The Contractor's test results will be accepted for pay if the required comparisons of data sets exceed 0.5%. The required comparisons of data shall be:

- 1. The Department's verification test results and the Contractor's verification test results will be compared using a paired sample t-test and F-test.
- 2. The Contractor' verification test results and the Contractor's quality control test results shall be compared using a t-test assuming equal variances and F-test.
 - If any of the above t-test and F-test analysis show that there is not more than 0.5% probability that the data sets match, then the Department's test data shall be used for determining Quality Levels and Pay Factors according to the methods in this Section.
- (c) Testing Schedule. Quality Control, Verification and Independent Assurance testing frequencies shall be in accordance with Table 106-1.
- (d) Reference Conditions. Three reference conditions can exist determined by the Moving Quality Level (MQL). The MQL will be calculated in accordance with the procedure in CP 71 for Determining Quality Level (QL). The MQL will be calculated using the Contractor's verification and quality control tests of asphalt content, gradation and in-place density. The MQL will be calculated on tests 1 through 3, then tests 1 through 4, then tests 1 through 5, then thereafter on the last five consecutive test results. The MQL will not be used to determine pay factors. The three reference conditions and actions that will be taken are described as follows:
 - 1. Condition green will exist for an element when an MQL of 90 or greater is reached, or maintained, and the past five consecutive test results are within the specification limits.

- 2. Condition yellow will exist for all elements at the beginning of production or when a new process is established because of changes in materials or the job-mix formula, following an extended suspension of work, or when the MQL is less than 90 and equal to or greater than 65. Once an element is at condition green, if the MQL falls below 90 or a test result falls outside the specification limits, the condition will revert to yellow or red as appropriate.
- 3. Condition red will exist for any element when the MQL is less than 65 or as described in subsection (b). The Engineer shall be notified immediately in writing and the Quality Control sampling and testing frequency increased to a minimum rate of 1/250 tons for that element. The Quality Control sampling and testing frequency shall remain at 1/250 tons until the MQL reaches or exceeds 78. If the MQL for the next five Quality Control tests is below 65, production will be suspended. After condition red exists, a new MQL will be started.

Production will remain suspended until the source of the problem is identified and corrected. Each time production is suspended, corrective actions shall be proposed in writing by the Contractor and approved in writing by the Engineer before production may resume.

Upon resuming production, the quality control sampling and testing frequency for the elements causing the condition red shall remain at 1/250 tons. If the QL for the next five process control tests is below 65, production will be suspended again.

- (e) Resolution of Disputes. The following procedure will be used to resolve disputes when F-test and t-test analysis show that the Contractor's verification test results and CDOT's verification test results are not from the same population:
 - The Engineer will quarter each verification sample into four equal parts. The Engineer will retain two
 parts, the Contractor shall take one part and the fourth part will be wasted. The Contractor will test one
 sample. The Engineer will test one part and mark the other part with the verification test number and
 store in a safe place.
 - 2. At any time during production, if there is a dispute concerning test results of an element, an analysis of the accumulated verification tests shall be performed. The analysis shall be a comparison of results from split samples of the verification tests using the paired sample t-test and F-test statistical methods on all verification tests that have been performed. If the analysis results in a value less than 5%,then a minimum of three samples from the splitting of verification samples that have been stored will be tested by an independent lab chosen by the Engineer. The lab performing independent assurance tests may be selected as the independent lab.
 - 3. The Department's Region Materials Engineer (RME) will review the analysis. If the RME determines that one lab's test results are closer to the independent lab results than the other, then the results of that lab will be used for pay factor calculations up to that point. If the RME can not determine that either lab is closer to the independent lab results, then another group of samples from the splitting of verification samples that have been stored will be tested by the independent lab. If this second analysis is inconclusive, then the Departments's verification test results will be used for pay factor calculations up to that point.

TABLE 106-1 SCHEDULE FOR MINIMUM SAMPLING AND TESTING

ELEMENT	CONTRACTOR QUALITY CONTROL	VERIFICATION TESTS	INDEPENDENT ASSURANCE TESTS
Asphalt Content	1/500 tons (first 2,500 tons, tests 1 to 5, splits to be tested by CDOT and Contractor). Then 1/500-ton substrata (8/4000-ton strata, 7 substrata samples independent of CDOT plus one substrata sample provided from a split of CDOT sample).	1/500 tons, first 2,500 tons. Then 1/4000-ton strata, split of each to be tested by CDOT and Contractor.	1/12,000 tons, By Region Materials Unit
In-Place Density	1/500 tons (first 2,500 tons, tests 1 to 5, at same spot and time by CDOT & Contractor). Then 1/500-ton substrata (2/1000 ton strata, 1 test independent of CDOT plus one substrata test taken at same spot & time as CDOT)	1/500 ton, first 2,500 tons. Then 1/1000-ton strata, each CDOT test to be tested by Contractor at same spot and time.	1/12,000 tons, By Region Materials Unit
Gradation	1/500 tons (first 2,500 tons, tests 1 to 5, splits to be tested by CDOT and Contractor). Then 1/1000-ton substrata (4/4000 ton strata, 3 substrata samples independent of CDOT plus one substrata sample provided from a split of CDOT sample).	1/500 tons, first 2,500 tons. Then 1/4000-ton strata, split of each to be tested by CDOT and Contractor.	1/12,000 tons, By Region Materials Unit
Air Voids and Voids in Mineral Aggregate	1/500 tons (first 2,500 tons, tests 1 to 5, splits to be tested by CDOT and Contractor). Then 1/1000-ton substrata (4/4000 ton strata, 3 substrata samples independent of CDOT plus one substrata sample provided from a split of CDOT sample).	1/500 tons, first 2,500 tons. Then 1/4000-ton strata, split of each to be tested by CDOT and Contractor.	1/12,000 tons, By Staff Materials Branch

Notes:
(1) For each process, the minimum number of verification tests (not including first 2,500 tons) will be at least 5 for asphalt content, gradation, air voids and voids in mineral aggregate. For in-place density the minimum number of verification tests will be 10.
(2) The minimum number of HBP compaction tests are those made after compaction has been completed and will be in addition to those made in Compaction Test Sections. The acceptance test result for each Compaction Test Section will be an average of the in-place density test results obtained by the Contractor's quality control tests in that Compaction Test Section.
(3) When unscheduled job mix formula changes are made (CDOT form 43) acceptance of the elements, except for in-place density, will be based on the actual number of samples that have been selected up to that time, even if the number is below the minimum listed in Note (1). Beginning with the new job mix formula, the quantity it represents shall be estimated. A revised schedule of quality control and verification tests will be based on that estimate. estimate.

- 98-1 I-76 Truck Study
- 98-2 HBP Pilot Void Acceptance Projects in Region 2 in 1997
- 98-3 1997 Hot Bituminous Pavement QC for Day Pilot Project with Void Acceptance
- 98-4 Hot Bituminous Pavement QC & QA Project Constructed in 1997 Under QPM2 Specification

REPORTS PUBLICATION LIST CDOT/CTI Research

- 96-1 Long-Term Performance Tests of Soil-Geosynthetic Composites
- 96-2 Efficiency of Sediment Basins: Analysis of the Sedient Basins Constructed as Part of the Straight Creek Erosion Control Project.
- 96-3 The Role of Facing Connection Strength in Mechanically Stabilized Backfill Walls
- 96-4 Revegetation of MSB Slopes
- 96-5 Roadside Vegetation Management
- 96-6 Evaluation of Slope Stabilization Methods (US-40 Berthod Pass) (Construction Report)
- 96-7 SMA (Stone Matrix Asphalt) Colfax Avenue Viaduct
- 96-8 Determinating Asphalt Cement Content Using the NCAT Asphalt Content Oven
- 96-9 HBP QC & QA Projects Constructed in 1995 Under QPM1 and QPM2 Specifications
- 96-10 Long-Term Performance of Accelerated Rigid Pavements, Project CXMP 13-006-07
- 96-11 Determining the Degree of Aggregate Degradation After Using the NCAT Asphalt Content Oven
- 96-12 Evaluation of Rumble Treatments on Asphalt Shoulders
- 97-1 Avalanche Forecasting Methods, Highway 550
- 97-2 Ground Access Assessment of North American Airport Locations
- 97-3 Special Polymer Modified Asphalt Cement (Final Report)
- 97-4 Avalanche Detection Using Atmospheric Infrasound
- 97-5 Keway Curb (Final Report)
- 97-6 IAUAC (Interim Report)
- 97-7 Evaluation of Design-Build Practice in Colorado (Pre-Construction Report)
- 97-8 HBP Pilot Void Acceptance Projects Completed in 1993-1996 (Interim Report)
- 97-9 QC & QA Projects Constructed in 1996 Under QPM2 Specifications (Fifth Annual Report)
- 97-10 Loading Test of GRS Bridge Pier and Abutment in Denver, CO
- 97-11 Faulted Pavements at Bridge Abutments

- 95-1 SMA (Stone Matrix Asphalts) Flexible Pavement
- 95-2 PCCP Texturing Methods
- 95-3 Keyway Curb (Contruction Report)
- 95-4 EPS, Flow Fill and Structure Fill for Bridge Abutment Backfill
- 95-5 Environmentally Sensitive Sanding and Deicing Practices
- 95-6 Reference Energy Mean Emission Levels for Noise Prediction in Colorado
- 95-7 Investigation of the Low Temperature Thermal Cracking in Hot Mix Asphalt
- 95-8 Factors Which Affect the Inter-Laboratory Repeatability of the Bulk Specific Gravity of Samples Compacted Using the Texas Gyratory Compactor
- 95-9 Resilient Modulus of Granular Soils with Fine Contents
- 95-10 High Performance Asphalt Concrete for Intersections
- 95-11 Dynamic Traffic Modelling of the I-25/HOV Corridor
- 95-12 Using Ground Tire Rubber in Hot Mix Asphalt Pavements
- 95-13 Research Status Report
- 95-14 A Documentation of Hot Mix Asphalt Overlays on I-25 in 1994
- 95-15 EPS, Flowfill, and Structure Fill for Bridge Abutment Backfill
- 95-16 Concrete Deck Behavior in a Four-Span Prestressed Girder Bridge: Final Report
- 95-17 Avalanche Hazard Index For Colorado Highways
- 95-18 Widened Slab Study

REPORTS PUBLICATION LIST CDOT/CTI RESEARCH

- 94-1 Comparison of the Hamburg Wheel-Tracking Device and the Environmental Conditioning System to Pavements of Known Stripping Performance
- 1-94 Design and Construction of Simple, Easy, and Low Cost Retaining Walls
- 94-2 Demonstration of a Volumenteric Acceptance Program for Hot Mix Asphalt in Colorado
- 2-94 The Deep Patch Technique for Landslide Repair
- 94-3 Comparison of Test Results from Laboratory and Field Compacted Samples
- 3-94 Independent Facing Panels for Mechanically Stabilized Earth Walls
- 94-4 Alternative Deicing Chemicals Research
- 94-5 Large stone Hot Mix Asphalt Pavements
- 94-6 Implementation of a Fine Aggregate Angularity Test
- 94-7 Influence of Refining Processes and Crude Oil Sources Used in Colorado on Results from the Hamburg Wheel-Tracking Device
- 94-8 A Case Study of concrete Deck Behavior in a Four-Span Prestressed Girder Bridge: Correlation of Field Test Numerical Results
- 94-9 Influence of Compaction Temperature and Anti-Stripping Treatment on the Results from the Hamburg Wheel-Tracking Device
- 94-10 Denver Metropolitan Area Asphalt Pavement Mix Design Recommendation
- 94-11 Short-Term Aging of Hot Mix Asphalt
- 94-12 Dynamic Measurements or Penetrometers for Determination of Foundation Design
- 94-13 High-Capacity Flexpost Rockfall Fences
- 94-14 Preliminary Procedure to Predict Bridge Scour in Bedrock (Interim Report)

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